

City of Lloydminster Sanitary Sewer Master Plan Final Report

September 2024



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4015 7 Street SE, Calgary AB T2G 2Y9, T: 403.254.0544 F: 403.254.9186

September 10, 2024

Our Reference: 28114

City of Lloydminster 4420 50 Avenue Lloydminster, AB/SK T9V 0W2

Attention: James Rogers, P.Eng., Senior Manager, Capital Infrastructure

Dear J. Rogers:

#### Reference: City of Lloydminster Sanitary Sewer Master Plan – Final Report

Enclosed is the Final Report for the City of Lloydmindster's Sanitary Sewer Master Plan. We trust that it meets your needs.

The key objective of the Sanitary Sewer Master Plan is to complete a review of the City's existing sanitary sewer collection system and assess its capacity to convey the current and future growth sanitary flow volumes effectively.

The Sanitary Sewer Master Plan will provide the City of Lloydminster with direction on infrastructure implementation and associated timelines to service future growth, while ensuring infrastructure remains fully functional in providing an appropriate level of service. This information will aid in making informed decisions on capital projects and will provide solutions for efficient, economic, and sustainable municipal services to residents and businesses.

We sincerely appreciate the opportunity to undertake this project on behalf of the City of Lloydminster. Should you have any questions or concerns, please do not hesitate to contact the undersigned at 403.254.5044

Sincerely,

Geoffrey Schulmeister, P.Eng., SCPM General Manager, Water and Environment





# **Corporate Authorization**

This document entitled "Sanitary Sewer Master Plan" has been prepared by ISL Engineering and Land Services Ltd. (ISL) for the use of City of Lloydminster. The information and data provided herein represent ISL's professional judgment at the time of preparation. ISL denies any liability whatsoever to any other parties who may obtain this report and use it, or any of its contents, without prior written consent from ISL.

Sarah Barbosa, P.Eng., ENV SP Project Lead

Geoffrey Schulmeister, P.Eng., SCPM Project Manager





# **Territory Acknowledgement**

## **City of Lloydminster**

The City of Lloydminster acknowledges that we are located on Treaty 6 Territory, and the City of Lloydminster respects the histories, languages, and cultures of First Nations, Metis, Inuit, and all First Peoples of Canada, whose presence continues to enrich our vibrant community.

## **ISL Engineering and Land Services Ltd.**

ISL Engineering and Land Services Ltd. acknowledges that our Calgary office and work takes place on the ancestral, traditional, and present-day territory of the Treaty 7 Nations of Southern Alberta. The confluence of the Bow and Elbow Rivers has been an important meeting place for Indigenous peoples since time immemorial, and we honour the Siksika, Piikani, and Kainai Nations of the Blackfoot Confederacy, the Bearspaw, Chiniki, and Goodstoney First Nations of the Stoney Nakoda Nations, and the Tsuut'ina Nation. We also acknowledge that this is the homeland of the Métis Nation of Alberta, Region 3.





# **Executive Summary**

## **E1.0 Introduction**

The City of Lloydminster (City) retained ISL Engineering and Land Services Ltd. (ISL) to complete a review of its existing sanitary sewer collection system and assess its capacity to convey the current and future growth sanitary flow volumes effectively. A review and assessment of both the condition and capacity of the sanitary sewer collection system was conducted to generate an updated Sanitary Sewer Master Plan (SSMP).

## E2.0 Report Summary

The overall SSMP is summarized as follows:

- **Purpose and scope of the Sanitary Sewer Master Plan (SSMP):** The SSMP is a comprehensive review and assessment of the existing and future sanitary sewer collection system in the City of Lloydminster. It aims to inventory and analyze the existing infrastructure, calibrate and update the hydraulic model, prepare capacity assessments, develop servicing plans, and provide a framework for future capital planning.
- **Study area:** The study area covers 24 neighbourhoods and approximately 23.5 quarter sections of recently annexed land, with a total area of about 5,870 ha. The study area is divided by the Alberta/Saskatchewan border and is located within the North Saskatchewan River Basin. The development type is classified by several land use districts, such as residential, commercial, industrial, and public service.
- **Population horizons and growth projections:** The SSMP considers six population horizons for the existing and future sanitary sewer collection system assessment, ranging from 2021 to 2051. The growth populations are based on an annual growth rate of 2.2%, a target population allocation of 70% to Alberta and 30% to Saskatchewan, and the anticipated development timelines for each area. The growth projections also account for residential densification, employment population, and land use densities.
- Design criteria and level of service: The SSMP uses the existing Sanitary Sewer Master Plan (AECOM, 2016), the City's Municipal Development Standards, and typical municipal servicing standards as the sources for the design criteria. The level of service for the sanitary sewer collection system is based on a Type 4 Huff design storm with a 1:25 year 24-hour return period. The performance of the sanitary sewer collection system is assessed in terms of peak discharge relative to pipe capacity and maximum hydraulic grade line elevation relative to ground.
- Existing sanitary sewer collection system and hydraulic model development: The existing sanitary sewer collection system consists of approximately 193 km of sanitary sewers, mostly made of PVC or VCT, with diameters ranging from 100 mm to 1350 mm. The sanitary sewer collection system has several trunk sewers that convey flows to the Wastewater Treatment Plant. The hydraulic model was constructed from scratch using the City's GIS data, record drawings, and assumptions as necessary. The model was calibrated using flow monitoring and rainfall data for both dry and wet weather conditions.
- Existing sanitary sewer collection system assessment and proposed capacity upgrades: The existing sanitary sewer collection system assessment identified areas of concern where the sanitary sewer collection system is surcharged or over capacity under different design storm scenarios. The assessment also estimated the inflow and infiltration rates based on the monitored and modelled data.

i





The proposed capacity upgrades include pipe upsizing, slope adjustments, and flow redirections at thirteen (13) locations in the system. The upgrades aim to resolve the capacity constraints and provide a sufficient level of service for the existing population. Seven (7) upgrades related to condition of pipes are also recommended.

- Future sanitary sewer collection system assessment and proposed concepts: Two future servicing concepts are developed, with a recommendation provided as to a preferred concept. System assessments were completed to ensure performance of existing sewers under future development conditions. Costing is also provided for future concepts as well as existing system upgrades as well as proposed staging.
- **Capital Planning:** Based upon recommended upgrades and provided staging, a proposed capital planning table is included noting upgrades and recommended timelines for implementation for consideration in the City's overall capital plan.

## **E3.0 SSMP Conclusions**

Conclusions for the sanitary sewer collection system are as follows:

- Significant inflow and infiltration was identified at Sites 7, 9, 12, and 14. Most other sites were within typical design guidelines of 0.28 L/s/ha or less.
- Flow monitoring and rain gauge data collected in 2022 indicated that no major rainfall events occurred during the monitoring period, as all storms were less than a 1:2 year return period.
- DWF and WWF calibration was undertaken for the 2022 monitoring period. Successful calibration was achieved based on agreement between the observed and modelled data.
- The sanitary sewer collection system was evaluated in terms of various return period events, with the level of service event identified as the 1:25 year design storm event.
- Thirteen (13) locations were flagged as areas of concern under the level of service event.
- A CCTV inspection program was undertaken in various areas of the city, covering a range of sewer installation years, materials, and pipe diameters. Pipe conditions were documented and flagged for repairs or maintenance depending on their condition rating.
- A risk assessment was undertaken to prioritize the capacity and condition upgrades recommended under existing sanitary sewer collection system conditions.
- The proposed sanitary sewer collection system is comprised of gravity sewers, lift stations, and forcemains to service future growth areas.
  - Two concepts were evaluated to service future areas: Concept 1 and Concept 2. The main difference between the two concepts is the ultimate tie-in point of one single-family quarter section on the west side of the city.
  - In Concept 1 it is tied-into the existing sanitary infrastructure and in Concept 2 it is routed to the proposed South Trunk.
  - Concept 1 was selected as the recommended option as it allows greater flexibility regarding staging of the quarter section.
- Under Ultimate Boundary Growth conditions, the existing sanitary sewer collection system was generally found to perform adequately.
  - Some surcharging was noted in seven (7) areas across the two concepts, however, in many cases the surcharging was minor.
  - Upgrades for three (3) of the seven (7) noted areas were recommended.





- The future network assumes all the recommended existing sanitary sewer collection upgrades are implemented. Therefore, these upgrades should be completed prior to any substantial densification or future development.
- Hydraulic assessment of the proposed sanitary sewer collection system is sufficient in managing sewage generated from the future development areas given that all proposed upgrades are implemented.

# **E4.0 SSMP Recommendations**

Recommendations for the sanitary sewer collection system are as follows:

- Prioritize upgrades to the existing sanitary sewer collection system based on the order documented in Table 6.11.
  - To mitigate surcharging in the existing sanitary sewer collection system, implement the necessary upgrades recommended under the LOS design storm scenario.
  - Remediate the pipes flagged as being in poor condition from the CCTV inspections.
- Continue condition assessments and flow monitoring and aligning infrastructure upgrades with development and roadworks programs to minimize costs.
- Conduct an inflow-infiltration field investigation program to pinpoint the sources of I-I. Field investigation could consist of smoke testing, micro flow monitoring, dye testing and CCTV inspections.
  - Once the field investigation is completed and areas of sources of I-I have been identified, these issues should be mitigated.
  - Additional flow monitoring and WWF calibration would be required following the program to update the runoff and I-I model parameters.
- Preferred servicing future concept recommended is Concept 1. Recommended staging of Concept 1 for the 5-, 10-, and 20-Year Growth Horizons are shown in Figures 7.12 to 7.14, respectively.
  - The future sanitary sewer collection system should be designed based on the City's Municipal Development Standards (City of Lloydminster, 2020).
- The SSMP should be reviewed and updated after significant periods of growth or every five years to update the hydrodynamic model and analysis with any capital upgrades completed by the City, and the most up-to-date growth plans. This could provide clarity on the planned location of development, the density of the proposed development, and the potential corresponding upgrades. This will ensure capacity is maintained and staging upgrades are advancing as needed.





# **Table of Contents**

1.0	Introd	duction	1
	1.1	Authorization	1
	1.2	Background	1
	1.3	Purpose of Study	1
2.0	Study	/ Area	2
	2.1	Location	2
	2.2	Development Type	2
	2.3	Population Horizons	8
	2.4	Growth Projections	9
3.0	Desig	gn Criteria	14
	3.1	Level of Service	14
	3.2	Assessment Criteria	15
	3.3	Future System Design Criteria	17
	3.4	Wet Weather Flow Component	18
4.0	Exist	ing Sanitary System	19
	4.1	Sanitary Collection System	.19
5.0	Hvdr	aulic Model Development	26
	5.1	Computer Model	
	5.2	Model Set-Up	26
	5.3	Catchment Delineation	29
	5.4	Flow Monitoring	34
	5.5	Model Calibration	34
6.0	Exist	ing System Assessment	45
	6.1	Inflow and Infiltration Review	45
	6.2	Capacity Assessment	49
	6.3	Condition Assessment	61
	6.4 6.5	RISK Assessment	/1
	0.5	Existing Samary System Opgrades Cost Estimates	.75
7.0	Futur	e Sanitary System	76
	7.1	Future System Concept Development	76
	7.2	Future System Concept Cost Estimates	80
	7.3 7.4	Future System Assessment	.01
	7.5	Staging of Preferred Servicing Concept	93
8.0	Capit	al Plan	97
•	0		00
9.0	Conc	Susions and Recommendations1	03
	9.1 0.2	Sanitary System Conclusions	104
	J.Z		103
10.0	Refe	rences1	06





#### **APPENDICES**

Appendix A Dry Weather Flow Diurnals
Appendix B Dry Weather Flow Calibration Graphs
Appendix C Wet Weather Flow Calibration Graphs
Appendix D Longitudinal Profile
Appendix E CCTV Inspection Reports
Appendix F CCTV Inspection Videos (SharePoint Link)
Appendix G Risk Assessment
Appendix H Cost Estimates





#### TABLES

Table 2.1:	Land Use District Descriptions	8
Table 2.2:	Population Horizon Assessment Scenarios	8
Table 2.3:	Incremental 3-Year Horizon (2025) Development Areas and Populations	10
Table 2.4:	Incremental 5-Year Horizon (2027) Development Areas and Populations	10
Table 2.5:	Incremental 10-Year Horizon (2032) Development Areas and Populations	11
Table 2.6:	Incremental 20-Year Horizon (2042) Development Areas and Populations	11
Table 2.7:	Incremental Ultimate Boundary Horizon (2051) Development Areas and Populations	11
Table 4.1:	Existing System Diameter Summary	19
Table 4.2:	Existing System Material Summary	20
Table 4.3:	Existing System Installation Period Summary	20
Table 5.1:	Minimum Design Slopes for Sewers	27
Table 5.2:	Manning's 'n' Roughness Coefficient	29
Table 5.3:	Sub-Catchment Population Calculation Parameters	30
Table 5.4:	Sub-Catchment Summary	30
Table 5.5:	Existing Sanitary Trunks and Upstream Collection Areas	33
Table 5.6:	Flow Monitoring Sites and Catchment Summary	34
Table 5.7:	Dry Weather Flow Calibration Period Summary	36
Table 5.8:	Dry Weather Flow Calibration Results	38
Table 5.9:	Wet Weather Flow Calibration Period Summary	40
Table 5.10:	Wet Weather Flow Calibration Results – Primary Event	43
Table 5.11:	Wet Weather Flow Calibration Results – Validation Event	44
Table 6.1:	Observed I-I Rates Based on 2022 Flow Monitoring Data	45
Table 6.2:	Modelled I-I Rates Based on Huff Design Storm Model Simulations	46
Table 6.3:	Existing System Proposed Capacity Upgrades	49
Table 6.4:	Pipe Condition Rating Summary	61
Table 6.5:	Pipe Material Expected Useful Life	64
Table 6.6:	Condition Assessment Summary	65
Table 6.7:	Generalized Condition Assessment Summary by Decade	67
Table 6.8:	Existing System Proposed Condition Upgrades	69
Table 6.9:	Existing System Upgrades Risk Assessment – Risk Criteria and Scoring	71





Table 6.10:	Existing System Upgrades Risk Assessment – Criteria Ranking	73
Table 6.11:	Existing System Upgrades Risk Assessment Priority Summary	74
Table 6.12:	Cost Estimates for Recommended Existing System Upgrades	75
Table 7.1:	Minimum Design Slopes for Sewers	76
Table 7.2:	Cost Estimates for Recommended Future Sanitary Servicing Concepts	80
Table 7.3:	Areas of Concern and Recommended Upgrades	82
Table 7.4:	Cost Estimates for Recommended Upgrades under Growth Conditions	92
Table 8.1:	Capital Planning Horizons	98





#### FIGURES

Figure 2.1:	Study Area	3
Figure 2.2:	Existing Neighbourhoods	4
Figure 2.3:	Topography	5
Figure 2.4:	Watershed Boundaries	6
Figure 2.5:	Existing Land Use	7
Figure 2.6:	2021 Population Projection Comparison	9
Figure 2.7:	Future Land Use	12
Figure 2.8:	Future Development Area Staging	13
Figure 3.1:	1:25 Year 24-Hour Q4 Huff Storm Rainfall Hyetograph	14
Figure 3.2:	Comparison of 24-hour Q4 Huff Storm Return Periods	15
Figure 4.1:	Existing Pipe Diameter	22
Figure 4.2:	Existing Pipe Material	23
Figure 4.3:	Pipe Installation Period	24
Figure 4.4:	Trunk Sewers	25
Figure 5.1:	Flow Split and Survey Locations	28
Figure 5.2:	Trunk Sewer Collection Areas	32
Figure 5.3:	Flow Monitor Sites and Catchments	35
Figure 5.4:	Sanitary Generation Rates	39
Figure 5.5:	2022 IDF Curve with Major Events Superimposed	41
Figure 6.1:	Inflow-Infiltration Rate Summary	48
Figure 6.2:	Existing 1:5 yr 24-hr Results	51
Figure 6.3:	Existing 1:5 yr 24-hr Spare Capacity	52
Figure 6.4:	Existing 1:10 yr 24-hr Results	53
Figure 6.5:	Existing 1:10 yr 24-hr Spare Capacity	54
Figure 6.6:	Existing 1:25 yr 24-hr Results	55
Figure 6.7:	Existing 1:25 yr 24-hr Spare Capacity	56
Figure 6.8:	Longitudinal Profile Key Plan	57
Figure 6.9:	Proposed Capacity Upgrades	58
Figure 6.10:	Existing Upgrades 1:25 yr 24-hr Results	59
Figure 6.11:	Existing Upgrades 1:25 yr 24-hr Spare Capacity	60





Figure 6.12:	2016 Condition Assessment Recommendations	62
Figure 6.13:	2023 Condition Assessment Proposed Locations	63
Figure 6.14:	2023 Condition Assessment	66
Figure 6.15:	Estimated Condition by Installation Year and Material	68
Figure 6.16:	Proposed Pipe Condition Upgrades	70
Figure 7.1:	Future Servicing Concept 1	78
Figure 7.2:	Future Servicing Concept 2	79
Figure 7.3:	Future 1:25 yr 24-hr Results – Concept 1	83
Figure 7.4:	Future 1:25 yr 24-hr Results Spare Capacity – Concept 1	84
Figure 7.5:	Future 1:25 yr 24-hr Results – Concept 2	85
Figure 7.6:	Future 1:25 yr 24-hr Results Spare Capacity – Concept 2	86
Figure 7.7:	Proposed Future Capacity Upgrades	87
Figure 7.8:	Future 1:25 yr 24-hr Results – Concept 1 Upgrades	88
Figure 7.9:	Future 1:25 yr 24-hr Results Spare Capacity – Concept 1 Upgrades	89
Figure 7.10:	Future 1:25 yr 24-hr Results – Concept 2 Upgrades	90
Figure 7.11:	Future 1:25 yr 24-hr Results Spare Capacity – Concept 2 Upgrades	91
Figure 7.12:	Future Sanitary Servicing Concept – 5-Year Staging	94
Figure 7.13:	Future Sanitary Servicing Concept – 10-Year Staging	95
Figure 7.14:	Future Sanitary Servicing Concept – 20-Year Staging	96
Figure 8.1:	Capital Plan Overview	.101
Figure 8.2:	Capital Planning Horizons	.102





#### ACRONYMS

Acronym	Description
AEPA	Alberta Environment and Protected Areas
ASP	Area Structure Plan
CCTV	closed-circuit television
City	City of Lloydminster
CIWEM	Chartered Institution of Water and Environmental Management
CN	Canadian National
CONC	concrete
CPR	Canadian Pacific Railway
DWF	dry weather flow
GIS	geographic information system
HGL	hydraulic grade line
ICI	industrial, commercial, institutional
IDF	intensity duration frequency
-	inflow and infiltration
ISL	ISL Engineering and Land Services Ltd.
Lidar	light detection and ranging
LOS	level of service
LP	longitudinal profile
McGill's	McGill's Industrial Services
No.	number
PVC	polyvinyl chloride
Q/Q <sub>man</sub>	peak discharge relative to sewer capacity
QA/QC	quality assurance/quality control
RDII	rainfall dependent inflow-infiltration
SSMP	Sanitary Sewer Master Plan
STL	steel
VCT	vitrified clay tile
WaPUG	Wastewater Planning Users Group
WWF	wet weather flow
WWTP	Wastewater Treatment Plant





## UNITS

Unit	Description
\$	dollars
%	percentage
ha	hectares
hr	hour
km	kilometre
L/ha/d	litres per hectare per day
L/p/d	litres per person per day
L/s	litres per second
L/s/ha	litres per second per hectare
m	metre
m/s	metres per second
mm	millimetre
mm/hr	millimetre per hour



# **1.0** Introduction

## 1.1 Authorization

The City of Lloydminster (City) retained ISL Engineering and Land Services Ltd. (ISL) to complete a review of its existing sanitary sewer collection system and assess its capacity to convey the current and future growth sanitary flow volumes effectively. A review and assessment of both the condition and capacity of the sanitary sewer collection system was conducted to generate an updated Sanitary Sewer Master Plan (SSMP).

## 1.2 Background

The SSMP was most recently updated in 2016. Since then, the city limits have been expanded via the 2022 Annexation Lands along with various sanitary sewer collection system upgrades or replacements being completed. The expected additional sanitary flows driven by expansion through annexation and population growth of the city, in conjunction with the normal deterioration of pipe condition and system upgrades that have occurred since 2016, are sufficient reasons to require this updated SSMP.

The updated SSMP will help the City understand the implications of servicing new developments by understanding each area's servicing approach and constraints. By completing a comprehensive review of the available background data and sanitary sewer collection system hydraulic model, maintaining consistent approaches to issues, and using sound engineering principles, while all the time protecting the natural and human environment, the updated SSMP will guide effective infrastructure improvement and expansion. The updated SSMP will also examine the capacity of the sanitary sewer collection system to determine the extent of upgrades required to maintain an appropriate level of service for existing and future residents and businesses.

## 1.3 Purpose of Study

The purpose of developing an updated SSMP is outlined as follows:

- Inventory and analyze the existing infrastructure under existing conditions;
- · Convert and update the City's existing hydraulic model;
- Calibrate the City's hydraulic model under dry weather flow (DWF) and wet weather flow (WWF) conditions to accurately represent the flow conditions within the City's existing sanitary sewer collection system;
- Use the calibrated hydraulic model to prepare capacity assessments of the existing sanitary sewer collection system under current and future growth conditions;
- Develop servicing plans for future growth. Locations and timing may be dependent on the following:
  - Availability of sufficient servicing needs;
  - Annexed land locations; and
  - Community planning;
- Determine what upgrades are required to the existing sanitary sewer collection system based on condition and capacity assessments and recommend future servicing options; and
- Provide a framework for future sanitary sewer collection system capital planning, including cost estimates and possible staging of infrastructure installations.



# **2.0** Study Area

### 2.1 Location

The city is divided by the Alberta/Saskatchewan border and is located approximately 250 km east of the City of Edmonton. The city is bordered by the County of Vermilion River No. 24 on the Alberta side and both the Rural Municipalities of Britannia No. 502 and Wilton No. 472 on the Saskatchewan side. The Yellowhead Highway (Highway 16) is an interprovincial highway that connects Manitoba to British Columbia through Lloydminster and is known as 44 Street/Ray Nelson Drive within city limits. Highway 17 runs north/south through Lloydminster along the Alberta/Saskatchewan border and is known as 50 Avenue within city limits. The study area is shown in Figure 2.1.

The study area encompasses 24 neighbourhoods, as well as approximately 23.5 quarter sections of recently annexed land (shown in Figure 2.2). Not all existing neighbourhoods are fully developed; therefore, future growth is anticipated both within these neighbourhoods and within the recently annexed land. The study area encompasses a total area of approximately 5,870 ha.

The highest elevation areas within city limits are approximately 670 m in elevation and located within the northwest and southwest corners of the city. The lowest elevation area within city limits is the northeast corner of the city at an elevation of approximately 615 m. The topography of the study area is shown in Figure 2.3.

The study area is located almost entirely within the Central North Saskatchewan River Watershed with the southwest corner of the study area adjacent to the boundary of the Battle River Watershed. Both watersheds are part of the North Saskatchewan River Basin, which is part of the Nelson-Churchill (Hudson Bay) Continental Drainage Basin. A map of the watershed boundaries is shown in Figure 2.4.

## 2.2 Development Type

The development type influences sanitary effluent generation rates and diurnal patterns; therefore, obtaining an appropriate classification was vital in ensuring that an accurate representation of the City's sanitary sewer collection system could be achieved. When determining development classifications for existing areas within the city, a land use district shapefile provided by the City was used.

A land use district map for existing development is illustrated in Figure 2.5, while Table 2.1 summarizes all land use district codes and their corresponding descriptions. The land uses were compared to aerial maps and Google Street View to confirm that parcels were properly categorized. For the purposes of the project, many of these land use districts were grouped together to form an overall land use. In this manner, the City was classified more broadly by several unique development types, including residential, commercial, industrial, and public service.





-----+ Railway Study Area









Study Area

## Neighbourhood

Airport Aurora Bud Miller Park Lakeland College **Central Business District** College Park East Lloydminster **Exhibition Association Golf Course Cemetery** Glenn E. Neilson Industrial Park Hill Industrial Husky Industrial Lakeside Landfill Larsen Grove North Industrial North Lloydminster Parkview Estates Sask Industrial Southridge **Steele Heights** The Willows Wallacefield West Commercial West Lloydminster Wigfield Industrial **Recently Annexed** 





\* Maximum Elevation Point Minimum Elevation Point

- Major Contour 10m Interval
- Minor Contour 2m Interval
- Study Area

# Elevation (m)



High : 676.38

Low: 615.174

Note: LiDAR data was acquired on 2019-04-18







Study Area **Existing Land Use** R1: Single-Detached Residential R2: Semi-Detached Residential R3: Row House Residential R4: Medium-Density Residential R5: High-Density Residential **RMH: Residential** Manufactured Home C1: Central Commercial C2: Highway Corridor Commercial C3: Neighbourhood Commercial C5: Service Commercial 11: Light Industrial 12: Medium Industrial **PS: Public Services** PU: Public Utility UP: Urban Park MA1: Municipal Airport Airside UT: Urban Transition DC1: Direct Control 1 DC2: Direct Control 2 DC3: Direct Control 3 DC4: Direct Control 4 DC5: Direct Control 5 DC6: Direct Control 6 DC7: Direct Control 7

Annexed Area-Urban Transition



FIGURE 2.5 EXISTING LAND USE LLOYDMINSTER SANITARY SEWER MASTER PLAN







District Code	District Description	District Code	District Description		
R1	Single-Detached Residential	C5	Service Commercial		
R2	Semi-Detached Residential	l1	Light Industrial		
R3	Row House Residential	12	Medium Industrial		
R4	Medium-Density Residential	PS Public Services			
R5	High-Density Residential	PU	Public Utility		
RMH	Residential Manufactured Home	Up	Urban Park		
C1	Central Commercial	MA1 Municipal Airport Airsid			
C2	Highway Corridor Commercial	UT	Urban Transition		
C3	Neighbourhood Commercial	DC (1-7)	Direct Control (1-7)		

#### Table 2.1: Land Use District Descriptions

### 2.3 **Population Horizons**

The City's sanitary sewer collection system was assessed for six (6) scenarios as summarized in Table 2.2.

Seenaria	Voor	Cumulative Population		
Scenano	rear	Alberta	Saskatchewan	Total
Existing Conditions <sup>2</sup>	2021	19,739	11,843	31,582
3-Year Growth	2025	22,081	12,570	34,651
5-Year Growth	2027	22,475	13,658	36,132
10-Year Growth	2032	23,564	17,584	41,148
20-Year Growth	2042	37,085	20,185	57,271
Ultimate Boundary	2051	46,461	20,688	67,149

#### Table 2.2: Population Horizon Assessment Scenarios

<sup>1</sup> The growth year scenarios are based on the year at the start of the project, which is 2022.

<sup>2</sup> The population for the existing conditions scenario is based on the 2021 Census (Statistics Canada, 2022).

The growth populations were initially determined by applying an annual growth rate of 2.2%. This growth rate was kept consistent with that applied within the design of the City's Wastewater Treatment Plant (WWTP). This was based on the City of Lloydminster and County of Vermilion River Joint Regional Growth Study (Applications Management, et.al, 2019). The target population allocation of 70% to the Alberta side of the city and 30% to the Saskatchewan side of the city was used to scale the populations for the future development areas.

Staging of growth areas was then refined by the City to align with the anticipated growth horizon for each future development area. This results in a non-linear growth rate that deviates from the annual growth rate of 2.2% that was initially applied across the city.





Additionally, the 3-Year Growth Horizon incorporates a densification population of 1,579. This population is based on a densification rate of 5% applied to the existing population obtained from the 2021 Census. This densification rate is also outlined within the Joint Regional Growth Study as part of the 2016 Draft Land Demand Generators Summary (Applications Management, et.al, 2019).

Growth did not occur at the rate anticipated within the 2019 Joint Regional Growth Study between 2019 and 2021; therefore, the population growth assumptions were reset to reflect the actual 2021 Census population (Statistics Canada, 2022). A comparison of these population projections is shown in Figure 2.6.



Figure 2.6: 2021 Population Projection Comparison

## 2.4 Growth Projections

Residential population estimates were generated based on the 2021 reset population projections, as well as net developable areas stipulated within local Area Structure Plans (ASPs), the 2013 Comprehensive Growth Strategy, the 2019 Joint Regional Growth Study, and the 2020 Annexation Application. These were applied on a persons/ha basis for the residential future development areas.

Approximately one and a half (1.5) quarter sections of future residential area were added to the anticipated growth areas between 75 Avenue and the city's west limit. The densities for these areas were based on the equivalent populations specified within the City's Municipal Development Standards for low-density and medium-density residential. It should be noted that this results in a deviation from the target population allocation of 70% to the Alberta side of the city and 30% to the Saskatchewan side of the city.





Employment population estimates were generated based on the 2019 Joint Regional Growth Study and interpolated for each of the population horizon assessment scenarios. These were applied on an employees/ha basis for the non-residential future development areas. Future development areas and land use classifications are shown in Figure 2.7.

As previously noted, staging of the City's future development areas was refined by the City to align with the anticipated development timelines for each area.

The growth considered in the City's development areas is summarized incrementally in Tables 2.3 to 2.7 for each of the growth horizons. Staging of the development areas by growth horizon is presented in Figure 2.8.

District Code	District Description	Area	Area Population		
District Code		ha	Residential	Employment	
RES-SF	Single-Family Residential	52.29	1,402	0	
RES-MF	Multi-Family Residential	1.45	88	0	
CBD	Commercial Business District	25.38	0	309	
IND	Industrial	137.17	0	1,668	
PS	Public Services	7.74	0	98	
	Total	224.03	1,490	2,074	

#### Table 2.3: Incremental 3-Year Horizon (2025) Development Areas and Populations

As noted above, the 3-Year Growth Horizon also incorporates residential densification based on a densification rate of 5% applied to the existing population within existing residential areas. This is equal to a densification population of 1,579 in addition to the growth population in the future development areas noted in Table 2.4.

#### Table 2.4: Incremental 5-Year Horizon (2027) Development Areas and Populations

District Code	District Description	Area	Population		
District Code	District Description	ha	Residential	Employment	
RES-SF	Single-Family Residential	40.80	1,158	0	
RES-MF	Multi-Family Residential	2.12	324	0	
CBD	Commercial Business District	44.52	0	544	
IND	Industrial	45.25	0	545	
PS	Public Services	21.90	0	288	
	Total	154.59	1,481	1,376	



District Code	District Description	Area	Population	
		ha	Residential	Employment
RES-SF	Single-Family Residential	156.26	4,250	0
RES-MF	Multi-Family Residential	13.69	614	0
CBD	Commercial Business District	53.95	60	709
IND	Industrial	148.40	0	1,992
PS	Public Services	2.65	92	0
	Total	374.95	5,016	2,700

#### Table 2.5: Incremental 10-Year Horizon (2032) Development Areas and Populations

#### Table 2.6: Incremental 20-Year Horizon (2042) Development Areas and Populations

District Code	District Description	Area	Population	
		ha	Residential	Employment
RES-SF	Single-Family Residential	434.48	11,770	0
RES-MF	Multi-Family Residential	53.29	4,353	0
CBD	Commercial Business District	69.63	0	957
IND	Industrial	247.05	0	3,453
PS	Public Services	0.00	0	0
Total		804.45	16,123	4,410

#### Table 2.7: Incremental Ultimate Boundary Horizon (2051) Development Areas and Populations

District Code	District Description	Area	Population	
		На	Residential	Employment
RES-SF	Single-Family Residential	399.19	9,878	0
RES-MF	Multi-Family Residential	0.00	0	0
CBD	Commercial Business District	175.70	0	2,137
IND	Industrial	378.30	0	4,578
PS	Public Services	0.00	0	0
	Total	953.19	9,878	6,715

The Municipal Development Plan only specifies residential areas, not the density of those areas. In the 2042 to 2051 timeframe to reach the Ultimate Horizon, there is not any other detailed information on the differentiation between single-family and multi-family residential parcels, so all were assigned as single-family residential. Though the increase in population density would result in a higher sewage generation, this is offset by a decrease in inflow and infiltration, as there are fewer services to multi-family residential properties compared to single-family residential properties. Therefore, as the exact split is unknown between single-family and multi-family residential parcels, it is assumed that the difference between dry weather and wet weather flows would be negated for this purpose.





# Future Land Use

Commercial Business District Industrial Public Service

- Medium Density Residential
- Low Density Residential

Note: Parcels shown are those to be serviced.





Legend Study Area Staging 3-Years 5-Years 10-Years 20-Years Ultimate

Note: Map does not include any vacant parcels under existing conditions that would be developed in the future, or densification to existing development.







# **3.0** Design Criteria

The design criteria used to assess the City's sanitary sewer collection system were derived from the existing Sanitary Sewer Master Plan (AECOM, 2016), the City's Municipal Development Standards, and typical municipal servicing standards in the Province of Alberta and the Province of Saskatchewan. In addition, sanitary effluent generation rates were derived based on the City's population rates, service areas, and historic sanitary influent at the City's WWTP.

## 3.1 Level of Service

To properly consider level of service (LOS), it was necessary to consider what the required LOS is in terms of Wet Weather Flow (WWF) in the City's sanitary sewer collection system. The LOS that was applied when assessing the sanitary sewer collection system for existing and future system assessments is summarized below.

The existing and future sanitary sewer collection system capacity was assessed based on a Type 4 Huff design storm with a 1:25 year 24-hour return period. A Huff rainfall distribution replicates a storm with a moderate peak intensity following an initial wetting period to ensure system response to the peak, which is ideal for sanitary sewer collection system analysis. In the case of the 1:25 year 24-hour Q4 Huff storm, the peak intensity is 10.96 mm/hr for a duration of 1.2 hours or 72 minutes, while the total rainfall depth produced over the entire duration is 67.44 mm. The rainfall hyetograph for this event is shown in Figure 3.1.



Figure 3.1: 1:25 Year 24-Hour Q4 Huff Storm Rainfall Hyetograph





A comparison of the 1:5 year and 1:10 year 24-hour Q4 Huff design storm events is shown below in Figure 3.2 for reference on scale. The 1:50 year storm is also shown in Figure 3.2 for comparison as this is a common storm frequency used to determine existing and future sanitary sewer collection system capacity.



Figure 3.2: Comparison of 24-hour Q4 Huff Storm Return Periods

## 3.2 Assessment Criteria

The performance of the sanitary sewer collection system under existing conditions is ultimately determined based on the available freeboard between the ground elevation and the high-water level elevation, which is represented by the maximum hydraulic grade line (HGL) at each manhole for each assessment design storm. Based on this, the maximum allowable surcharge in the gravity portion of the sanitary sewer collection system must remain at least 2.5 m from the ground surface during a design storm scenario. The exception to this is in the case of shallow sewers with less than 2.5 m of cover. In these instances, the allowable surcharge should not exceed the obvert/crown of the pipe. The performance of the sanitary sewer collection system was assessed in terms of two (2) relationships: the Maximum HGL Elevation Relative to the Ground and the Peak Discharge Relative to Sewer Pipe Capacity as follows:





### 3.2.1 Maximum HGL Elevation Relative to the Ground

Maximum HGL elevation relative to the ground elevation is the amount of freeboard between the maximum water elevation and the ground elevation at each manhole when maximum flow passes through.

Hence, the maximum HGL elevation relative to the ground elevation with a value of:

- Greater than 0.00 m is denoted as a red dot, indicating surcharge/back-up to surface;
- Between -2.40 m and 0.00 m is denoted as an orange dot (maximum HGL peaks within 2.4 m below the ground elevation, indicating possible basement back-ups);
- Between -3.50 m and -2.40 m is denoted as a yellow dot (maximum HGL peaks within 2.4 m and 3.5 m below the ground elevation, indicating no basement back-ups but possibly an elevated HGL); and
- Less than -3.50 m is denoted as a green dot (maximum HGL peaks 3.5 m below the ground elevation).

#### 3.2.2 Peak Discharge Relative to Sewer Pipe Capacity

Peak discharge relative to pipe capacity indicates the ratio of peak flow to pipe capacity in wet weather conditions; as a corollary to this, the data can be interpreted to indicate the amount of spare capacity during peak flows. This is calculated by taking the ratio of the modelled flow in a pipe and its corresponding capacity. Pipes with ratios higher than 1.00 are considered to have no spare capacity, thus indicating a section of pipe that may require upgrading, particularly where the length of the section is long enough to cause surcharge conditions immediately in the upstream reach.

Hence, the peak discharge relative to sewer capacity with a ratio of:

- Greater than 1.20 is denoted with a red line (significantly over capacity);
- Between 1.00 and 1.20 is denoted as an orange line (over capacity, or in another words the capacity is diminishing as the maximum flow theoretically occurs at roughly 93% of the pipe's diameter. This means that in principle, sewers with a Q/Q<sub>man</sub> [peak discharge relative to pipe capacity] ratio equal to or less than 1.05 have their flow still contained within the pipe's diameter);
- Between 0.86 and 1.00 is denoted as a yellow line (less than 14% of spare capacity available); and
- Less than 0.86 is denoted as a green line (spare capacity available).

Both relationships should be evaluated in conjunction to pinpoint any potential capacity deficiencies in the system. For example:

- The maximum HGL elevation relative to the ground elevation with a value that is between -2.40 m and 0.00 m (an orange dot) may indicate a location with a possible basement back-up; however, the peak discharge relative to pipe capacity ratio at the same location could have a value of less than 0.86 (a green line), indicating the pipe is not surcharged. This could suggest a relatively shallow pipe.
- The ratio of peak discharge relative to pipe capacity for forcemains is always above 1.00, as these operate under pressurized conditions by nature. This should not be of any concern, as this color coding for gravity sanitary sewers is not intended for this application.





In addition to these two (2) scenarios, the spare capacity of each pipe was determined. This indicates the amount of additional flow each pipe can convey before it becomes completely utilized. The amount of spare capacity ranges from less than 0 L/s to over 100 L/s, with the least capacity illustrated in red and the most capacity illustrated in green. In determining the spare capacity, it becomes evident which pipes are available to convey any additional flows due to future development and which pipes should remain untouched.

Existing sanitary sewer forcemains should be analyzed, and should be sized in the future to maintain a minimum velocity of 1.0 m/s or reaching 1.0 m/s at least two (2) times daily. The velocity should not exceed a velocity of 3.0 m/s, with the preferred velocity being 2.5 m/s.

## 3.3 Future System Design Criteria

### 3.3.1 Dry Weather Flow Generation Rates

The Dry Weather Flow (DWF) generation rates applied to the growth scenarios were employed from the City's Municipal Development Standards (2020) as follows:

- Residential Areas 320 L/p/d; and
- Non-Residential Areas 0.2 L/s/ha or 17,280 L/ha/d.

Although actual generation rates in the existing sanitary sewer collection system may be less than those outlined above for some areas, the use of these rates is a more conservative approach for future design over adopting historic generation trends, which are subject to change.

It should be noted that the employment population was provided in Sections 2.3 and 2.4 for reference; however, the non-residential sanitary effluent generation rates are applied on an area basis in keeping with the City's Municipal Development Standards.

## 3.3.2 Peaking Factors

Peaking factors for the future sanitary sewer collection system were calculated in accordance with the City's Municipal Development Standards. These include the following:

• Peaking factor derived based on Harmon's formula for residential areas:

$$PF = 1 + \frac{14}{4 + P^{\frac{1}{2}}}$$

- Where, P is the design contributing population in thousands; and
- PF must be at least 2.5; and
- Peaking factor for non-residential areas is 3.0.

Although actual peaking factors in the existing sanitary sewer collection system may be less than those outlined above for some areas, the use of these rates is a more conservative approach for future design over adopting historic peaking trends, which are subject to change.





## 3.4 Wet Weather Flow Component

A constant inflow and infiltration (I-I) allowance of 0.28 L/s/ha per the City's Municipal Development Standards (2020) was applied to each growth catchment to simulate the wet weather response. This is in alignment with the Alberta Environment and Protected Areas (AEPA) Standards and Guidelines for Municipal Waterworks, Wastewater, and Storm Drainage Systems for new development.

For modelling purposes, the average variable hydrograph was obtained by adjusting the RTK parameters on top of the WWF calibrated model such that the ensuing average modelled I-I rate of 0.28 L/s/ha was obtained for the future sanitary sewer collection system. The RTK method is discussed in detail in Section 5.5.





# **4.0** Existing Sanitary System

## 4.1 Sanitary Collection System

Lloydminster is currently serviced by approximately 187 km of sanitary sewers. The sanitary sewer collection system details with regards to pipe diameter, material, and installation period are shown in Figures 4.1, 4.2, and 4.3, respectively. The sanitary sewers are predominantly made of polyvinyl chloride (PVC) or vitrified clay tile (VCT). Pipe diameters range from 100 mm to 1,350 mm, with most being between 200 mm and 250 mm. Tables 4.1 to 4.3 below summarize the sanitary sewer collection system based on diameter, material, and installation year, respectively.

Diameter	Total Length	Percentage of Total	
mm	m	%	
100	195	0.10	
150	882	0.47	
200	93,995	50.33	
250	23,771	12.73	
300	12,462	6.67	
375	13,578	7.27	
400	453	0.24	
450	11,938	6.39	
500	206	0.11	
525	3,867	2.07	
600	6,322	3.39	
675	850	0.46	
750	4,833	2.59	
900	3,087	1.65	
1050	6,621	3.54	
1200	2,773	1.48	
1350	356	0.19	
Unknown	578	0.31	
Total	186,768	100.00	

#### Table 4.1: Existing System Diameter Summary





# Table 4.2: Existing System Material Summary

Matorial	Total Length	Percentage of Total
Material	m	%
Concrete (CONC)	31,475	16.85
Polyvinyl Chloride (PVC)	75,537	40.44
Steel (STL)	7	0.00
Vitrified Clay Tile (VCT)	78,939	42.27
Polyethylene (PE)	459	0.25
Unknown	351	0.19
Total	186,768	100.00

#### Table 4.3: Existing System Installation Period Summary

Installation Pariod	Total Length	Percentage of Total
	m	%
1940-1949	8,478	4.54
1950-1959	5,916	3.17
1960-1969	17,895	9.58
1970-1979	37,339	19.99
1980-1989	37,350	20.00
1990-1999	14,075	7.54
2000-2009	36,866	19.74
2010-2021	28,775	15.41
Unknown	74	0.04
Total	186,768	100.00

The existing sanitary sewer collection system's primary trunk sewers are shown in Figure 4.4 and summarized below:

#### • 25 Street Trunk:

- This trunk is along 25 Street from 59 Avenue to 47 Avenue, conveying flows from the 25 Street Trunk collection area to the Southeast Trunk.
- 36 Street Trunk:
  - This trunk is along 36 Street from 52 Avenue to 40 Avenue, conveying flows from the 25 Street Trunk, 36 Street Trunk, and Southeast Trunk collection areas to the East Trunk.
- 47 Street Trunk:
  - This trunk is along 46 Street from 45 Avenue to 43 Avenue, crosses the Legion Ball Park, then is along 47 Street from 40 Avenue to 37 Avenue, conveying flows from the 47 Street Trunk collection area to the East Trunk.
- 52 Street Trunk:
  - This trunk is along 52 Street from 50 Avenue to east of 37 Avenue, conveying flows from the 52 Street Trunk collection area to the East Trunk.
- 62 Street Trunk:
  - This trunk is along 62 Street from 63 Avenue to 53 Avenue, conveying flows from the 62 Street Trunk collection area to the West Trunk.




#### • East Trunk:

- This trunk is along 36 Street, 39 Avenue, 41 Street, 37 Avenue, and 44 Street, and follows the alignment of the East Drainage Channel from 40 Avenue and 36 Street to 67 Street.
- This trunk conveys flows from the 25 Street Trunk, 36 Street Trunk, 52 Street Trunk, Southeast, and East Trunk collection areas to the City's WWTP.
- North Trunk:
  - This trunk is along 49 Avenue and 67 Street from 62 Street to the East Trunk, conveying flows from the 62 Street Trunk and West Trunk collection areas to the East Trunk.
  - It should be noted that there is no existing collection area for the North Trunk, as these lands are currently undeveloped.
- Southeast Trunk:
  - This trunk is along 18 Street, 47A Avenue, 19 Street, 47 Avenue, 20 Street, 46 Avenue, 27 Street, 45 Avenue, and 33 Street from 52 Avenue and 18 Street to 45 Avenue and 36 Street.
  - This trunk conveys flows from the 25 Street Trunk and Southeast Trunk collection areas to the 36 Street Trunk.

#### • West Trunk:

- This trunk is along 70 Avenue, 59 Street, 53 Avenue, and 62 Street, and follows the alignment of the Northwest Drainage Channel from 70 Avenue and 43 Street to 49 Avenue and 62 Street.
- This trunk conveys flows from the 62 Street and West Trunk collection areas to the North Trunk.



Lege	nd
772	Study Area
•	Manhole
Pipe [	Diameter
	100 mm
	150 mm
<u> </u>	200 mm
	250 mm
	300 mm
	375 mm
	400 mm
	450 mm
	500 mm
	525 mm
	600 mm
	675 mm
	750 mm
	900 mm
	1050 mm
	1200 mm
	Linknown
	UNKNOWN



FIGURE 4.1 EXISTING PIPE DIAMETER LLOYDMINSTER SANITARY SEWER MASTER PLAN



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- Study Area
  - Manhole

## Pipe Material

- Concrete (CONC)
- Polyvinyl Chloride (PVC)
- Steel (STL)
- Vitrified Clay Tile (VCT)
- Polyethylene (PE)
- Unknown



FIGURE 4.2 EXISTING PIPE MATERIAL LLOYDMINSTER SANITARY SEWER MASTER PLAN







## Legend Study Area Manhole Installation Period 1940-1949 1950-1959 1960-1969 1970-1979 1980-1989 1990-1999 2000-2009

— 2010-2021 — Unkown



FIGURE 4.3 PIPE INSTALLATION PERIOD LLOYDMINSTER SANITARY SEWER MASTER PLAN





2.24	Study Area
•	Manhole
	Gravity Sewer
Trunk	Sewer
	25 Street Trunk
	36 Street Trunk
-	47 Street Trunk
	52 Street Trunk
_	62 Street Trunk
_	East Trunk
	North Trunk
	Southeast Trunk
	West Trunk



FIGURE 4.4 TRUNK SEWERS LLOYDMINSTER SANITARY SEWER MASTER PLAN









## **5.0** Hydraulic Model Development

#### 5.1 Computer Model

The computer model used to assess the City's sanitary sewer collection system was InfoWorks ICM by Autodesk. InfoWorks ICM is a powerful analysis tool that computes inflow from sewage generation rates and rainfall dependent inflow-infiltration and routes these sanitary flows through the hydraulic system network. Based on the hydraulic simulation, the model can be used to evaluate locations with surcharge or flooding conditions under various rainfall events. Sanitary sewer flows are also determined to identify over-capacity pipes based on peak flows and pipe capacities. The InfoWorks ICM software is significantly integrated with the ArcGIS platform, which was used to assist in the construction of the model.

### 5.2 Model Set-Up

The InfoWorks ICM model was built from scratch using the City's geographic information system (GIS) data, supplemented by record drawings, references to the previous XPSWMM model, and necessary assumptions. The model pipe network was developed from scratch to ensure the most up-to-date system data, consistent with the City's GIS information, was incorporated and all revisions or assumptions tracked for the City's reference. This was also done to ensure new or upgraded infrastructure not previously included was accounted for, while eliminating duplicate network elements.

The pipe network data was first processed in ArcGIS to remove duplicate entries and combine pipe segments with matching NAMENUM shapefile fields to simplify the pipe network for importing into InfoWorks ICM. This also reduces the number of artificial nodes required in the model. Any abandoned or inactive network elements were filtered out as these would not be needed in the model. Once this process was complete, the pipe network and associated infrastructure was imported into InfoWorks ICM for verification.

One of the critical steps as part of this project was to ensure proper connectivity of the system, and review elevations, diameters, and slopes to determine if the inputted data appeared accurate. This process was completed by producing longitudinal profiles (LPs) of every pipe network in the city. For the purposes of system verification, the LPs were used to identify:

- Missing data:
  - Connectivity errors;
  - Missing pipes or nodes; and
  - Reversed pipe direction;
- Potentially erroneous pipe gradients:
  - Flat slopes;
  - Steep slopes; and
  - Adverse slopes;
- Inconsistent profiles:
  - Upstream invert of downstream pipe above downstream invert of upstream pipe;
  - Two (2) pipes with identical elevations in series; and
  - Suspicious pipe drops.





Identified issues were remediated though the request and review of the available record drawings. Survey was undertaken in the fall of 2022 for flow split locations that were not located in most upstream manholes in the system and where record drawings were not available to prioritize downstream flow splits for model calibration purposes. All flow split and survey locations are shown in Figure 5.1. Where record drawings were not available or did not provide the required information, additional survey was undertaken at prioritized locations in the network or assumptions were applied when no other information was available. Any updates to the pipe network information, including the sources of new data and any assumptions made, were tracked in the network element properties. The network element status was flagged as 'modified' in the model's user text fields. Missing information and pipe assumptions included:

- Missing downstream invert information was taken from the downstream neighbouring pipes; and
- Missing upstream invert information was calculated based on the City's minimum design slope for each pipe size, as stipulated in Table 5.1.

Nominal Pipe Diameter	Minimum Design Slope
mm	%
200	0.40
250	0.28
300	0.22
375	0.15
450	0.12
525	0.10
≥ 600	0.10

#### Table 5.1: Minimum Design Slopes for Sewers

Where manhole rim elevations were missing from the City's GIS information, the rim elevation was extracted from the City's 2019 Light Detection and Ranging (LiDAR) surface. Many of the manholes were missing sump elevation data; therefore, the minimum neighbouring pipe invert was assumed as the manhole sump elevation. Like the pipe network elements, the node status was set to "modified" where any assumptions were applied. For those manholes that did not have a manhole diameter provided, a manhole area was applied based on the diameter of the connecting pipes. All manholes with missing diameters have connecting pipes of diameter less than or equal to 675 mm; therefore, a manhole diameter of 1.2 m was assumed for all manholes with missing diameters.

Artificial nodes were added as needed, primarily where connections exist along the pipe rather than at the manhole. Additional artificial nodes and artificial links or pipes were added mainly at the lift stations as needed. Artificial nodes were considered to have zero area.



- Study Area
- ---- Existing Pipe
- Existing Manhole Not Surveyed (1,893)
- Existing Manhole Surveyed (12)
- Existing Flow Split Surveyed (49)
- Existing Flow Split Not Surveyed (6)
- Existing Upstream Flow Split Surveyed (1) 0
- Existing Upstream Flow Split Not Surveyed (78) •



FIGURE 5.1 FLOW SPLIT AND SURVEY LOCATIONS LLOYDMINSTER SANITARY SEWER MASTER PLAN

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Table 5.2:	Manning's 'n' Roughness Coefficient	
	Material	Manning's 'n' Coefficient
	Concrete	0.013
	Corrugated Steel Pipe	0.024
	Polyvinyl Chloride	0.011
	Steel	0.016
	Vitrified Clay Tile <sup>1</sup>	0.017
	Unknown	0.013

Manning's coefficients for pipe roughness were assigned as outlined in Table 5.2.

Table 5.2	Manning's	'n' Roughness	Coefficient
	Marining 3	II I touginicaa	Oberneient

<sup>1</sup> This roughness coefficient assumes that the pipe's integrity remains and that broken pieces are not laying in the flow channel.

#### 5.3 Catchment Delineation

Following the set-up of the physical sanitary sewer collection system model, it was necessary to delineate the City into sub-catchments for the purpose of generating DWFs and WWFs. The sub-catchments were delineated based on a lot-by-lot basis and land use classification.

The legal shapefile provided by the City was used as a baseline to derive the sub-catchments. A working copy of the shapefile was clipped such that only the parcels that are serviced by the City's sanitary sewer collection system are included. This generally required removing parcels outside of the city boundary as well as undeveloped land parcels.

The remaining parcels underwent a visual inspection to remove any parcels not requiring sanitary service. This included parcels such as utility easements, parks, transportation corridors, and vacant land. Visual inspection was completed through a combination of reviewing the available aerial imagery, the base mapping provided within ArcGIS, Google Maps, and Google Street View.

The existing land use is based on the zoning shapefile provided by the City and was used to assign land use classifications to the sub-catchments developed from the legal shapefile. The inspection process noted above also involved reviewing parcels with unclassified land uses, such as Direct Control parcels. Using Google Maps and its pinned location tags, these parcels were also classified.

To assign residential populations to each parcel, densities and unit densities were assigned based on the existing land use shapefile provided by the City. All Single-Detached Residential and Semi-Detached Residential parcels were assumed to have one unit per lot. The unit count for all other residential classification parcels greater than 0.5 ha in area varies by land use. Unit counts were either based on available unit information for buildings, determined from aerial imagery, or based on the calculation of design populations as outlined in the City's Municipal Development Standards (2020). The parameters used to calculate the populations for each sub-catchment by land use classification are summarized in Table 5.3.



	Density	Unit Density	Population Density		
	units/ha	capita/unit	capita/ha		
Single-Detached Residential	-	3.5	-		
Semi-Detached Residential	-	3.5	-		
Row House Residential	37.5	2.4	90.0		
Medium-Density Residential	25.0	2.4	60.0		
High-Density Residential	74.0	2.4	177.6		
Residential Manufactured Home	-	2.4	-		
Direct Control 1 <sup>1</sup>	-	3.5	-		

#### Table 5.3: Sub-Catchment Population Calculation Parameters

<sup>1</sup> Direct Control 1 is included as it is the only Direct Control land use classification that includes dwellings as a permitted use.

These populations were scaled to match the 2021 Census populations (Statistics Canada, 2022). A summary of the sub-catchment areas and populations by land use type is provided in Table 5.4.

#### Area **Calculated Population Scaled Population** Land Use Type ha capita capita Single-Detached Residential 27,675 506.90 21,641 Semi-Detached Residential 16.26 1,092 862 Row House Residential 16.34 1,366 1,108 Medium-Density Residential 71.09 8,259 6,509 **High-Density Residential** 2.92 473 382 **Residential Manufactured Home** 18.56 744 582 **Central Commercial** 28.24 0 0 Highway Corridor Commercial 126.66 0 0 0 Neighbourhood Commercial 2.49 0 0 0 Service Commercial 15.62 167.92 0 0 Light Industrial Medium Industrial 376.04 0 0 0 0 **Public Services** 113.29 Urban Park 0 0 99.67 0 **Urban Transition** 24.36 0 **Direct Control 1** 17.65 654 500 **Direct Control 2** 6.53 0 0 **Direct Control 3** 14.18 0 0 **Direct Control 5** 3.74 0 0 0 0 **Direct Control 6** 1.31 **Direct Control 7** 7.29 0 0 31,586<sup>1</sup> Total 1.637.04 40,261

#### Table 5.4: Sub-Catchment Summary

<sup>1</sup> The scaled population is slightly larger than the existing population of 31,582 noted in Table 2.2 in Section 2.3, obtained from the 2021 Census (Statistics Canada, 2022), due to rounding of the scaled population for each parcel.





Sub-catchments were also digitized to represent road polygons. Including roads was necessary to account for I-I for WWF calibration. To accomplish this, polygons of the road networks were created by using the void space of the existing land use shapefile provided by the City. A validation process was undertaken to remove any green spaces or utility easements. This shapefile was then clipped such that only roads with sanitary sewers buried underneath were accounted for, as these would be the sections contributing to I-I. Finally, the shapefile was divided into Thiessen polygons for each manhole within the City.

Sub-catchment connections to the pipe network were primarily preprocessed in ArcGIS based on the sanitary service/lateral information provided by the City. Each sub-catchment that intersected a service connection was then spatially joined to the closest pipe to that service connection. As each pipe stipulates the upstream and downstream manhole IDs, this information was then transferred back into each sub-catchment.

The sub-catchments were assumed to tie-into the upstream node, as this provides a slightly more conservative modelling approach. The exception to this was at critical flow split locations, where the flow split within the manhole defined the catchment areas for each flow monitoring location. At these locations, sub-catchments were assigned to the downstream node to ensure flow contributions were properly allocated within each flow monitor catchment. If sub-catchments were assigned to upstream nodes at critical flow split locations, the flows might be inaccurately routed in the wrong direction, potentially leading to either overestimation or underestimation of flows in each flow monitoring catchment, depending on invert elevations.

Sub-catchment connections were imported into the model following the preprocessing step. A visual inspection of the sub-catchment connections was then undertaken for quality assurance/quality control (QA/QC) to correct any connections that appeared erroneous due to automating the process in ArcGIS. Any sub-catchments that were not assigned a tie-in node as part of the preprocessing due to missing services were also connected to the network at this point.

Collection areas for the City's existing sanitary sewer trunks are shown in Figure 5.2. These trunks and their corresponding upstream collection areas are summarized in Table 5.5.



- Study Area Manhole
  Gravity Sewer
  Trunk Sewer
  25 Street Trunk
  36 Street Trunk
  47 Street Trunk
  52 Street Trunk
- 62 Street Trunk
- East Trunk
- North Trunk
- Southeast Trunk
- West Trunk

### **Trunk Net Collection Area**



25 Street Trunk Collection Area 36 Street Trunk Collection Area 47 Street Trunk Collection Area 52 Street Trunk Collection Area 62 Street Trunk Collection Area East Trunk Collection Area North Trunk Collection Area Southeast Trunk Collection Area West Trunk Collection Area





## Table 5.5: Existing Sanitary Trunks and Upstream Collection Areas

Tauak	Upstream Collection Area											
типк	25 Street	36 Street	47 Street	52 Street	62 Street	East	North	Southeast	West			
25 Street	Yes											
36 Street	Yes	Yes						Yes				
47 Street			Yes									
52 Street				Yes								
62 Street					Yes							
East	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes			
North					Yes		Yes		Yes			
Southeast	Yes							Yes				
West					Yes				Yes			

Note: The upstream collection areas encompass all areas that are located upstream of any portion of the trunk. For example, the most downstream section of the East Trunk is downstream of all collection areas.





## 5.4 Flow Monitoring

BotCorp was engaged to install 15 flow monitors at strategic locations within city limits to assist in developing realistic sewer flows in the model. Flow monitoring occurred between the beginning of June and the end of July 2022.

The flow monitoring data was used in conjunction with the City's rain gauge data from two (2) active locations during the flow monitoring period in the area to allow model calibration for both dry and wet weather conditions based on flows and rainfall. The flow monitoring sites and catchment areas are illustrated in Figure 5.3, noting that catchment areas include roadways and green spaces in addition to residential and non-residential parcels, including some areas that would not be contributing DWFs. The flow monitoring sites and catchments are summarized in Table 5.6.

Site	Upstream Sites	Trunk Location	Net Catchment Area	Catchment Population
1	3, 4, 5, 6, 7, 8, 9, 10, 11	East	24.98	0
2	12, 13, 14, 15	North	301.31	835
3	4, 6, 7, 8, 9, 10, 11	East	89.31	1,982
4	6,10, 11	47 Street	88.97	2,525
5	N/A	52 Street	200.15	3,897
6	N/A	N/A	99.48	2,161
7	8, 9	Southeast	33.53	1,645
8	N/A	Southeast	121.56	5,184
9	N/A	25 Street	153.96	4,403
10	N/A	36 Street	77.71	1,821
11	N/A	N/A	49.49	1,778
12	N/A	N/A	80.43	3,236
13	N/A	West	145.43	1,794
14	12	N/A	40.63	0
15	12, 13, 14	West	155.22	327

#### Table 5.6: Flow Monitoring Sites and Catchment Summary

Flow monitoring and rainfall data was compiled for calibration of the InfoWorks ICM model.

## 5.5 Model Calibration

#### **Dry Weather Flow Calibration**

Following the hydraulic model construction and compilation of the flow monitoring data, calibration of the wastewater model was then initiated. Calibration was a crucial part of the project, to accurately represent flows under both dry and wet weather conditions.



- Study Area
- Node
- → Gravity Sewer

## Flow Monitor Locations

	Site 1
	Site 2
	Site 3
$\bigcirc$	Site 4
	Site 5
$\bigcirc$	Site 6
	Site 7
$\bigcirc$	Site 8
$\bigcirc$	Site 9
	Site 10
	Site 11
$\bigcirc$	Site 12
$\bigcirc$	Site 13
$\bigcirc$	Site 14
	Site 15



FIGURE 5.3 FLOW MONITOR SITES AND CATCHMENTS LLOYDMINSTER SANITARY SEWER MASTER PLAN









The first step in calibration is DWF calibration, where a period with minimal or no rainfall influence on the network was identified for each flow monitoring site. The weeks that were chosen to represent the sanitary sewer collection system under DWF conditions are summarized in Table 5.7.

DWF Period	Flow Monitor
	1
	2
	3
lune 2 10 2022	5
June 3-10, 2022	7
	8
	9
	10
	11
lung 4 11 2022	13
Julie 4-11, 2022	14
	15
huby 11 18 2022	4
July 11-10, 2022	6
July 21-28, 2022	12

#### Table 5.7:Dry Weather Flow Calibration Period Summary

Most of the sites were installed on June 3, 2022; therefore, the DWF period begins on this day. However, some sites were not installed until June 4, 2022; therefore, the DWF period for these sites starts one day later. Different weeks needed to be used for three (3) sites due to missing or erroneous data recorded during the flow monitoring period.

After the dry weather flow dates were deduced, it was necessary to establish residential and nonresidential diurnals. This first involved determining baseflows that generally represent infiltration to the sanitary sewer collection system. These baseflows are derived independently from the sanitary generation rates, providing a more conservative overall estimate to the sanitary flows in the sanitary sewer collection system. Baseflows were initially assumed to be 80% of minimum flows (typically nighttime flows) and were adjusted as needed to derive accurate diurnals.

Following the establishment of baseflows, to further proceed towards DWF calibration, diurnals were developed. Diurnals were derived by taking the difference between recorded flow rates and the determined baseflow, dividing this value by the average flow from each day, and deducing the average per hour. With this, weekday, Saturday, and Sunday diurnals were produced for all flow monitoring sites. Residential diurnals were developed for each site with non-residential diurnals developed for Sites 2, 5, 13, 14, and 15. Diurnals were adjusted slightly to meet the peak flows that were observed in the monitored data. A total of 20 diurnals were created; graphical representations of the diurnals can be found in Appendix A.





Once the baseflows and diurnals were defined, a combination of determination and adjustment of diurnals as well as identification and adjustment of dry weather sewage flow generation rates was undertaken. Dry weather flow sewage generation rates were estimated by considering the difference between the average flow rates and the defined baseflows, then taking the difference and dividing it by upstream residential populations and non-residential (commercial, industrial, institutional) areas based on anticipated flow rates.

Successful calibration results will produce volume and peak flow errors less than  $\pm 10\%$ , based on the Chartered Institution of Water and Environmental Management (CIWEM), formerly the Wastewater Planning Users Group (WaPUG), Code of Practice. Table 5.8 indicates that the recommended values were met at all sites. Graphical representations of the sites also indicate that there is good agreement between the observed and modelled data, and that the calibration results are sufficient.

A generalized peak flow for the monitored data was applied for many of the sites. This was done to remove any irregularities in the data that cannot be replicated in the model. These irregularities may be due to lift stations turning on/off at certain times, short periods of rainfall, or unusual increases in discharge from parcels at various times. One of the more notable occurrences of this is on June 5, 2022, at Site 5, where there was an approximately 20 L/s spike in flow recorded. Since this spike appears to be inconsistent with the rest of the flow monitoring data at this site, it was excluded for the purposes of calibrating the model. Otherwise, there is a solid agreement between modelled and monitored flow data under DWF conditions.

Final DWF comparison plots are provided in Appendix B and the final dry weather flow generation rates employed for the study are shown in Figure 5.4.

## Table 5.8: Dry Weather Flow Calibration Results

		Generalized	Generalized Minimum Baseflow Flow	Deset	Upstream	Unstream		0		Public		Peak Flow		Volume		
Flow Monitor	DWF Period	Minimum Flow		Flow Rate	ow Total ate Contributing Area	Total Contributing	Residential	Rate	Rate	Services Rate	Monitored	Modelled	Difference	Monitored	Modelled	Difference
		L/s	L/s	L/s/ha	ha	Population	L/p/d	m³/ha/day	m³/ha/day	m³/ha/day	L/s	L/s	%	m <sup>3</sup>	m³	%
1	June 3 - 10	75.00	22.00	0.88088	24.98	-	-	-	-	-	200.00	181.99	-9.90	84,056.6	81,129	-3.61
2	June 3 - 10	22.00	4.40	0.01460	301.31	835	200.00	5.00	3.00	1.00	70.00	65.27	-7.24	27,840.5	30,336	8.23
3	June 3 - 10	36.00	3.20	0.03577	89.31	1,982	150.00	5.00	5.00	1.00	120.70	111.91	-7.85	46,651.1	47,366	1.51
4	July 11 - 18	11.00	0.60	0.00674	88.97	2,525	325.00	5.00	-	1.00	23.61	22.08	-6.94	9,700.0	9,573	-1.33
5	June 3 - 10	17.00	13.60	0.06796	200.15	3,897	125.00	5.00	5.00	1.00	42.50	43.15	1.51	18,861.8	18721	-0.75
6	July 11 - 18	6.00	2.10	0.02111	99.48	2,161	165.00	5.00	5.00	1.00	17.00	15.79	-7.64	6,876.2	6,676	-3.00
7	June 3 - 10	21.00	5.70	0.17105	33.53	1,645	335.00	-	-	1.00	78.89	75.89	-3.96	28,035.9	30,851	9.13
8	June 3 - 10	5.50	3.30	0.02689	121.56	5,184	300.00	5.00	-	-	30.99	28.91	-7.18	9,397.4	10,352	9.22
9	June 3 - 10	6.00	3.60	0.02338	153.96	4,403	215.00	5.00	-	1.00	23.09	22.09	-4.53	8,287.5	8,946	7.36
10	June 3 - 10	3.00	2.40	0.03149	77.71	1,821	200.00	-	-	1.00	8.00	8.65	7.56	3,422.3	3,420	-0.07
11	June 4 - 11	3.00	2.40	0.04849	49.49	1,778	110.00	5.00	-	1.00	10.50	9.99	-5.14	3,625.8	3,686	1.64
12	July 21 - 28	3.75	2.06	0.02685	80.43	3,236	250.00	5.00	-	1.00	13.15	12.44	-5.70	4,929.1	5,367	8.15
13	June 4 - 11	2.00	0.60	0.00413	145.43	1,794	180.00	5.00	2.00	-	15.00	14.31	-4.84	4,512.0	4,940	8.66
14	June 4 - 11	6.50	0.69	0.01692	40.63	-	-	10.00	5.00	-	25.00	24.27	-3.01	8,921.9	9,906	9.93
15	June 4 - 11	13.50	1.25	0.00805	155.22	327	200.00	5.00	4.00	-	48.00	44.22	-8.55	18,747	18,797	0.27

## Legend:



Generalized Peak Flows

Within +/- 10% Error Margin

_	_	_	



Study Area

Manhole

----- Gravity Sewer

#### Flow Monitor Locations

$\bigcirc$	Site 1
	Site 2
	Site 3
	Site 4
	Site 5
$\bigcirc$	Site 6
	Site 7
	Site 8
$\bigcirc$	Site 9
	Site 10
	Site 11
	Site 12
	Site 13
	Site 14
	Site 15

#### **Residential Generation Rate**

100 - 149 L/p/d
150 - 199 L/p/d
200 - 249 L/p/d
250 - 299 L/p/d
300 - 350 L/p/d

#### Non-Residential Generation Rate

1 m <sup>3</sup> /ha/d
2 m <sup>3</sup> /ha/d
3 m <sup>3</sup> /ha/d
4 m <sup>3</sup> /ha/d
5 m <sup>3</sup> /ha/d
10 m <sup>3</sup> /ha/d



FIGURE 5.4 DWF GENERATION RATES LLOYDMINSTER SANITARY SEWER MASTER PLAN







#### Wet Weather Flow Calibration

WWF calibration was undertaken to ensure the model was accurately representing the amount of inflow and infiltration (I-I) to the sanitary sewer collection system during wet weather events. To do so, wet weather periods, during which a response to wet weather was observed in the flow monitoring data, were established. The weeks that were chosen to represent the sanitary sewer collection system under WWF conditions are summarized in Table 5.9.

Primary WWF Period	Validation WWF Period	Flow Monitor
June 13-18, 2022	June 23-28, 2022	11
		1
		2
		3
		5
		7
luna 22 28 2022	lune 14 10, 2022	8
Julie 23-28, 2022	June 14-19, 2022	9
		10
		12
		13
		14
		15
July 5-10, 2022	June 14-19, 2022	4
July 5-10, 2022	June 28-July 3, 2022	6

#### Table 5.9: Wet Weather Flow Calibration Period Summary

For WWF, a primary event was selected for each site as the event that exhibited the most significant rainfall response. This was June 23, 2022, for most sites, except for three (3) sites – Sites 4, 6, and 11. Site 4 is missing data during this period and has Site 6 located upstream. The only other storm that works well for both sites is the peaky one on July 6, 2022, which is why this event was selected for these two (2) sites. For Site 11, the baseflow alignment between the DWF week and the June 23, 2022, event is not ideal. The event on June 15, 2022, has better alignment; therefore, this event was selected for this site.

It should be noted that all events captured during the flow monitoring period are considered less than a 1:2 year storm event based on the City's Intensity Duration Frequency (IDF) curve. The City's IDF curve with major events captured during the 2022 flow monitoring period superimposed shown in Figure 5.5.



FIGURE 5.5 2022 IDF CURVE WITH MAJOR EVENTS SUPERIMPOSED LLOYDMINSTER SANITARY SEWER MASTER PLAN









Calibrating the WWF in InfoWorks ICM was best achieved by using the RTK method. This method accounts for the effects of rainfall derived infiltration and inflow (RDII) entering the network. The RTK method assumes that there are three (3) hydrographs, each attributing to the RDII seeping into the sanitary sewer collection system. Each hydrograph consists of three (3) parameters, including:

- R Area under the graph representing the proportion of rainfall that enters the sanitary sewer collection system
- T The time it takes the rainfall to reach the peak of the hydrograph
- K The ratio between time to recession and time to peak

Three (3) sets of RTK parameters were defined for each site, representing short-, medium-, and long-term rainfall responses to aid in the calibration of the model under WWF conditions.

Following successful calibration of the primary event, the validation event was used to assess the accuracy of the calibration. The calibration's RTK parameters were adjusted to better align with the validation event; however, the primary event calibration was still favoured. Tables 5.10 and 5.11 provide a summary of the primary and validation event calibration results, respectively. The RTK parameters for each site are also outlined in Tables 5.10 and 5.11.

There were two (2) sites (Sites 12 and 13) where the error margin was minimally exceeded for the primary event. Increasing the volume to achieve the CIWEM criteria would throw off the validation results further, given the volume errors for these two (2) sites are on the high end for the validation event. Since they are marginally exceeded for the primary event, they were left as is so that the validation event is better matched. Additionally, the monitoring data for Site 13 on June 27, 2022, does not follow the anticipated flow pattern. This may be a result of a temporary blockage, which is not replicated in the model results.

Final WWF comparison plots for the primary and validation event calibrations are provided in Appendix C.



## Table 5.10: Wet Weather Flow Calibration Results – Primary Event

Parameter		Site 1	Site 2	Site 3	Site 4	Site 5	Site 6	Site 7	Site 8	Site 9	Site 10	Site 11	Site 12	Site 13	Site 14	Site 15
R1		0.03	0.012	0.001	0.011	0.018	0.0105	0.0575	0.019	0.037	0.01	0.005	0.035	0.005	0.02	0.001
T1		10	5	3	0.5	0.5	0.5	0.35	1.5	2	1	2	0.25	0.1	4	0.5
K1		2	1	2	2	1.5	2	1	2	2	1	1.5	0.5	3	2	1
R2		0.25	0.015	0.001	0.001	0.009	0.025	0.115	0.009	0.007	0.005	0.025	0.018	0.006	0.043	0.001
Т2		5	4	3	5	5	5	1.5	5	5	0.5	3	3	2.5	3	4
K2		2	2	2	3	2	3	1	3	3	1	2	2	2	2.75	2
R3		0.15	0.001	0.001	0.007	0.03	0.045	0.03	0.04	0.04	0.015	0.03	0.05	0.013	0.037	0.001
ТЗ		15	12.5	10	15	15	15	15	15	15	9	10	15	15	12.5	12.5
КЗ		7	5	3	7	3	7	7	7	7	6	3	7	7	7	5
Monitored Peak Flow	L/s	472.42	176.21	284.76	39.26	101.15	24.58	255.18	50.58	98.15	27.55	15.00	83.95	29.51	100.14	116.60
Modelled Peak Flow	L/s	467.63	191.54	319.84	38.60	103.85	26.16	257.79	50.12	99.11	27.29	14.84	81.45	30.27	99.82	133.02
Difference	%	-1.03	8.00	10.97	-1.72	2.60	6.03	1.01	-0.92	0.97	-0.94	-1.06	-3.07	2.51	-0.31	12.34
Monitored Volume	m³	80,330	25,845	42,810	7,986	16,223	5,293	28,625	9,730	10,294	3,094	3,294	6,982	5,298	10,576	17,148
Modelled Volume	m <sup>3</sup>	76,969	29,560	45,926	8,215	17,437	5,747	31,221	9,495	10,157	3,329	3,619	6,298	4,714	11,024	18,660
Difference	%	-4.37	12.57	6.79	2.78	6.96	7.89	8.32	-2.47	-1.35	7.04	8.98	-10.86	-12.39	4.06	8.10

#### Legend:

Generalized Peak Flows Within Error Margin

Outside of Error Margin





## Table 5.11: Wet Weather Flow Calibration Results – Validation Event

Parameter		Site 1	Site 2	Site 3	Site 4	Site 5	Site 6	Site 7	Site 8	Site 9	Site 10	Site 11	Site 12	Site 13	Site 14	Site 15
R1		0.03	0.012	0.001	0.011	0.018	0.0105	0.0575	0.019	0.037	0.01	0.005	0.035	0.005	0.02	0.001
T1		10	5	3	0.5	0.5	0.5	0.35	1.5	2	1	2	0.25	0.1	4	0.5
K1		2	1	2	2	1.5	2	1	2	2	1	1.5	0.5	3	2	1
R2		0.25	0.015	0.001	0.001	0.009	0.025	0.115	0.009	0.007	0.005	0.025	0.018	0.006	0.043	0.001
Т2		5	4	3	5	5	5	1.5	5	5	0.5	3	3	2.5	3	4
K2		2	2	2	3	2	3	1	3	3	1	2	2	2	2.75	2
R3		0.15	0.001	0.001	0.007	0.03	0.045	0.03	0.04	0.04	0.015	0.03	0.05	0.013	0.037	0.001
ТЗ		15	12.5	10	15	15	15	15	15	15	9	10	15	15	12.5	12.5
КЗ		7	5	3	7	3	7	7	7	7	6	3	7	7	7	5
Monitored Peak Flow	L/s	367.57	170.20	170.20	32.50	64.03	24.13	141.68	36.05	51.63	15.06	17.07	37.48	22.77	59.51	79.14
Modelled Peak Flow	L/s	347.95	153.82	226.58	47.49	81.06	22.18	179.41	38.91	62.54	17.76	19.05	54.47	23.00	73.26	99.11
Difference	%	-5.64	-10.65	24.88	31.56	21.01	-8.82	21.03	7.34	17.44	15.22	10.42	31.20	0.99	18.77	20.14
Monitored Volume	m <sup>3</sup>	73,432	25,218	42,401	7,570	17,144	6,327	28,428	8,934	9,359	3,404	3,300	5,173	4,098	9,770	17,539
Modelled Volume	m <sup>3</sup>	83,252	31,914	50,009	10,730	18,634	5,726	34,244	10,432	11,535	3,568	3,336	7,081	5,028	12,258	20,240
Difference	%	11.80	20.98	15.21	29.45	8.00	-10.49	16.98	14.36	18.87	4.59	1.07	26.94	18.50	20.30	13.35

#### Legend:

Generalized Peak Flows Within Error Margin Outside of Error Margin





## **6.0** Existing System Assessment

## 6.1 Inflow and Infiltration Review

To estimate the I-I rate based on the monitored sewage flows in the sanitary sewer collection system, a review of the flow monitoring data was undertaken. The rates observed through flow monitoring are summarized in Table 6.1 below. The monitored I-I rates represent the measured peak WWF minus the measured peak DWF divided by the corresponding gross total upstream catchment area.

Flow Monitor	Peak DWF	Peak WWF	Difference	WWF/DWF Ratio	Gross Upstream Area	I-I Rate
Sile	L/s	L/s	L/s		ha	L/s/ha
1	200	472	272	2.36	1,319.26	0.21
2	70	176	106	2.52	759.15	0.14
3	120	285	165	2.37	1,034.45	0.16
4	22	33	10	1.47	509.80	0.02
5	42	101	59	2.41	259.84	0.23
6	17	24	7	1.42	392.51	0.02
7	79	255	176	3.23	406.99	0.43
8	30	51	21	1.69	159.86	0.13
9	23	98	75	4.25	201.14	0.37
10	9	28	19	3.06	89.92	0.21
11	10	19	9	1.87	66.12	0.13
12	14	84	70	6.09	107.97	0.65
13	14	30	16	2.11	187.00	0.08
14	25	100	75	4.01	54.74	1.37
15	47	117	70	2.48	404.40	0.17

Table 6.1: Observed I-I Rates Based on 2022 Flow Monitoring Data

The AEPA Standards and Guidelines for Municipal Waterworks, Wastewater, and Storm Drainage Systems (2012) recommends accommodating an I-I rate of no larger than 0.28 L/s/ha (design criterion for new development). Based on Table 6.1, it is apparent that this criterion was exceeded at Sites 7, 9, 12, and 14, which is not uncommon in older areas due to degraded pipes or connections that would no longer be permitted (e.g. weeping tiles or sump pumps discharging into the sanitary sewer collection system, as well as storm cross connections). For example, Sites 7 and 9 both have similar pipe installation years, so there could be a commonality there.

Table 6.2 summarizes the I-I rates established in the calibrated model for the 1:5, 1:10, 1:25, and 1:50 year design storm events. An I-I rate comparison of the monitored and modelled rates is illustrated in Figure 6.1.





	Average Peak I-I Rate										
Flow Monitor Site	1:5 Year	1:10 Year	1:25 Year	1:50 Year							
Cho	L/s/ha	L/s/ha	L/s/ha	L/s/ha							
1	0.58	0.68	0.82	0.92							
2	0.34	0.40	0.48	0.55							
3	0.56	0.66	0.79	0.89							
4	0.35	0.42	0.50	0.56							
5	0.47	0.55	0.66	0.74							
6	0.38	0.46	0.55	0.61							
7	0.91	1.08	1.29	1.45							
8	0.43	0.52	0.61	0.69							
9	0.70	0.83	1.00	1.12							
10	0.33	0.40	0.48	0.53							
11	0.53	0.62	0.74	0.83							
12	0.92	1.09	1.31	1.47							
13	0.22	0.26	0.31	0.36							
14	0.87	1.03	1.24	1.39							
15	0.37	0.43	0.52	0.59							
Average	0.53	0.63	0.75	0.85							

#### Table 6.2: Modelled I-I Rates Based on Huff Design Storm Model Simulations

The average monitored I-I rate calculated from the fifteen (15) flow monitoring sites in Table 6.1 is 0.29 L/s/ha, which is considerably lower than the modelled I-I rates. The I-I rates observed during flow monitoring are lower than the design storm assessment scenarios for all sites except for Site 14. When comparing the City's IDF curve with the major events observed during the flow monitoring period, it is noted that there was not a rainfall event with a significant return period. As previously mentioned, all the monitored events were less than a 2-year return period storm as shown in Figure 5.5. Therefore, it was expected that the monitored rates would be lower than the modelled rates because of the minimal rainfall during the flow monitoring period.

Site 14 is nearest to Rain Gauge 1 in the northwest; however, since rainfall is typically quite spatially variable, the rain gauge may not have captured more intense rain in the Site 14 catchment area. This could be one reason for the discrepancy between the monitored and modelled rates.





Another factor is the catchment area that is used to calculate these rates. Due to the large number of flow splits in the city, the catchment areas are based on using the lower pipe invert at flow split locations. This works well for low flow scenarios such as dry weather flows where the majority of flow is likely conveyed through the lower invert pipes. Under wet weather flow conditions, the flows are likely being split more significantly between any downstream pipes. This could skew the catchment areas for the flow monitoring sites and in turn skew the rate calculations. Therefore, it is recommended that these be used only to get a general sense of the I-I in the sanitary sewer collection system.

The rates generally highlight that the I-I for Sites 7, 9, 12, and 14 is particularly high, especially compared to AEPA's guideline of 0.28 L/s/ha. This presents a potentially significant risk to the existing sanitary sewer collection system if the city were to get a larger rainfall event in the future. This should provide guidance on where future I-I reduction programs may be focused on in the future and supports the need for these programs in the city.

#### **Inflow and Infiltration Reduction**

Remediation measures in the existing sanitary sewer collection system could reduce some of the capacity constraints by minimizing I-I. This could potentially resolve some of the capacity constraints without additional upgrades completed to the existing sanitary sewer collection system. A detailed I-I program could be undertaken to pinpoint areas with extreme I-I, which could involve several field tests and inspections, including the following.

Visual Inspections: Interior and exterior visual inspections of sanitary manholes are a quick and inexpensive way to identify obvious defects and assess the condition of joints, seals, and other possible I-I sources.

**Smoke Tests:** Non-toxic, odourless smoke is injected into sanitary sewer manholes. The locations of exiting smoke can indicate where I-I might enter the sanitary sewer collection system. Smoke will typically appear from roof drains, catchbasins or yard drains connected to the sanitary sewer collection system. The smoke may also appear from cracks in the pavement above the sewer, around homes with foundation drains connected to the sewer or in basements through sump pumps, floor drains or other direct openings to the sanitary sewer collection system.

**Dye Tests:** A non-toxic dye is added to a stormwater source upstream of suspected I-I locations. The stormwater can then be traced through the sanitary sewer collection system to confirm locations where stormwater is entering the sanitary sewer collection system. This tends to be most useful in locating cross connections, where smoke testing may not provide evidence of them.

**Targeted Closed-Circuit TV (CCTV) Inspections:** A video camera is sent through sections of the sanitary sewer collection system recording the condition of the pipes. The video footage can help identify I-I problem areas, such as cracks, root intrusions, leaks, and stormwater cross-connections.

Through this program, recommendations for mitigation measures would be proposed to establish an I-I reduction program. Following the completion of the I-I reduction program, additional flow monitoring and recalibration would be needed to determine if the mitigation measures were sufficient alone in alleviating capacity constraints, or if upgrades are still needed. Overall, this could potentially reduce the capital costs of upgrades. It should be noted that City is already planning to implement an I-I reduction program as a separate project from the SSMP.



FIGURE 6.1 INFLOW-INFILTRATION RATE SUMMARY LLOYDMINSTER SANITARY SEWER MASTER PLAN







## 6.2 Capacity Assessment

The capacity assessment results for the 1:5 year 24-hour Q4 Huff storm are illustrated in Figures 6.2 and 6.3 for peak discharge relative to pipe capacity ( $Q/Q_{man}$ ) and maximum HGL elevation relative to ground (Max HGL) and spare capacity, respectively. The capacity assessment results for the 1:10 year storm are illustrated in Figures 6.4 and 6.5. The capacity assessment results of the LOS rainfall event, i.e., the 1:25 year storm, are illustrated in Figures 6.6 and 6.7. Longitudinal profiles (LP) and corresponding numbers (No.) of the major trunks outlined in Figure 4.4 and areas with significant surcharge under the LOS rainfall event have also been provided in Appendix D. A longitudinal profile key plan for the surcharging areas is shown in Figure 6.8.

### 6.2.1 Existing System Capacity Upgrades

Based on the existing sanitary sewer collection system capacity assessment upgrades to improve areas of concern were developed. The identified upgrades are summarized in Table 6.3 and shown in Figure 6.9.

Upgrade No.	Name	Description	LP No.
		Upgrade from 200 mm sewer to 300 mm sewer along 32 Street from 45 Avenue to 46 Avenue.	1
1	1 Sewer	Upgrade from 200 mm sewer to 250 mm sewer along 46 Avenue from 31 Street close to 32 Street.	2
	opgradoo	Upgrade of 200 mm sewer to 250 mm sewer along 31 Street from 45A Avenue to 46 Avenue.	3
2	44 Street Sewer Upgrade	Upgrade from 200 mm sewer to 250 mm sewer along 44 Street from 54 Avenue to 56 Avenue.	5
3	51 Street Sewer Upgrade	Upgrade from 300 mm sewer to 375 mm sewer along 51 Street from 52 Avenue to 54 Avenue including pipe slope adjustments. New 375 mm sewer from 51 Street to 52 Street crossing the Canadian National (CN) Rail property and tracks along 52 Avenue.	6
4	52 Street Trunk Upgrade	Upgrade from 450 mm sewer to 525 mm sewer along 52 Street from 45 Avenue to 49 Avenue. Connection of 300 mm sewer along 50 Avenue to the existing 450 mm sewer along 52 Street.	7
5	48 Avenue Sewer Upgrade	Upgrade from 300 mm sewer to 375 mm sewer along 48 Avenue from 49 Street to 52 Street crossing the CN Rail tracks. Replacement of 375 mm pipe segments for slope adjustments.	8
6	36 Street Trunk Upgrade West	Upgrade from 375 mm sewer to 450 mm sewer along 36 Street from 47 Avenue to 49 Avenue.	9
7	36 Street Trunk Upgrade East	Upgrade from 750 mm sewer to 900 mm sewer along 36 Street from 40 Avenue to 45 Avenue.	10
8	East Trunk Upgrade	Upgrade from 750 mm sewer to 900 mm sewer along 36 Street and 41 Street as well as through the undeveloped land from 37 Avenue to 40 Avenue.	11

#### Table 6.3: Existing System Proposed Capacity Upgrades





Upgrade No.	Name	Description	LP No.
9	Southeast Trunk Upgrade	Upgrade from 375 mm and 450 mm sewer to 525 mm sewer along 46 Avenue/20 Street, 47 Avenue, and 47A Avenue/19 Street from 18 Street to 25 Street.	12
10	47 Avenue Sewer Upgrade	Upgrade from 250 mm sewer to 300 mm sewer along 47 Avenue from 23 Street to 25 Street.	13
11	53 Avenue Sewer Upgrade	Upgrade from 300 mm sewer to 375 mm sewer along 53 Avenue from 25 Street to 27 Street.	14
12	25 Street Trunk Upgrade	Upgrade from 300 mm sewer to 375 mm sewer along 59 Avenue and 25 Street from College Drive to 57B Avenue.	15
13	59 Avenue Sewer Upgrade	Upgrade from 450 mm and 525 mm sewer to 600 mm sewer along 59 Avenue from 50 Street to south of 55 Street crossing the CN Rail tracks.	16

The Aurora Sewer Upgrades (Upgrade 1) are proposed based on the elevated HGL highlighted in the model results. However, City Operations staff have not had any concerns reported in this area to date. Therefore, it is recommended that this location be monitored for operational issues and upgrades prioritized as needed.

The final alignment of the proposed 44 Street Sewer Upgrade (Upgrade 2) is to be determined during Detailed Design as there may be the option to redirect these sewer flows north along 55 Avenue and east along 45 Street as part of a potential Water and Sewer Replacement Program.

An alternative to the Southeast Trunk Upgrade (Upgrade 9) may be to redirect sanitary sewer flows northeast along 19 Street and through the undeveloped portion of Wallacefield. This has been considered as part of the future sanitary sewer collection system and is discussed in Section 7.1.

Although the results highlight capacity issues in the existing 250 mm pipe along 53 Avenue from 23 Street to 25 Street, no upgrades are proposed at this location. There is minimal surcharging occurring in this section based on the model results and the sewer depth is a minimum of 4 m below the ground surface. Additionally, the City installed a sewer overflow in this area in 2014 to reduce capacity issues. That said, the City noted that there may be a flow split upstream that is not captured in the City's as-built information, which may result in less flow being directed to the sewer overflow in the model than in reality. It is recommended that this be surveyed in the future for the City's records and to improve modelling accuracy in the area.

It should be noted that the 59 Avenue Sewer Upgrade (Upgrade 13) should be undertaken off-line to maintain the use of the existing sanitary sewer collection system during construction. Once this is complete, the existing pipe is to be abandoned or removed following the installation of the new sanitary sewer pipe as requested by the City.

Results from the existing sanitary sewer collection system assessment for the 1:25 year storm LOS event, including the upgrades noted above, are shown in Figures 6.10 and 6.11 with longitudinal profiles provided in Appendix D.





Study Area

# Maximum HGL Elevation Relative to Ground

- Less than -3.5m
- Between -3.5m and -2.4m
- Between -2.4m and 0.0m
- Greater than 0.0m

## Peak Discharge Relative to Pipe Capacity

- Less than 86%
- Between 86% and 100%
- Between 100% and 120%
- Greater than 120%





- Study Area
- Manhole

#### Pipe Spare Capacity

- Less than 0L/s
- 0 25L/s
- 25 50L/s
- 50 75L/s
- 75 100L/s
- Greater than 100L/s





Study Area

# Maximum HGL Elevation Relative to Ground

- Less than -3.5m
- Between -3.5m and -2.4m
- Between -2.4m and 0.0m
- Greater than 0.0m

## Peak Discharge Relative to Pipe Capacity

- Less than 86%
- Between 86% and 100%
- Between 100% and 120%
- Greater than 120%





- Study Area
- Manhole

#### Pipe Spare Capacity

- Less than 0L/s
- 0 25L/s
- 25 50L/s
- 50 75L/s
- 75 100L/s
- Greater than 100L/s





Study Area

# Maximum HGL Elevation Relative to Ground

- Less than -3.5m
- Between -3.5m and -2.4m
- Between -2.4m and 0.0m
- Greater than 0.0m

## Peak Discharge Relative to Pipe Capacity

- Less than 86%
- Between 86% and 100%
- Between 100% and 120%
- Greater than 120%





- Study Area
- Manhole

#### Pipe Spare Capacity

- Less than 0L/s
- 0 25L/s
- 25 50L/s
- 50 75L/s
- 75 100L/s
- Greater than 100L/s




- Study Area
- Manhole
- ----- Gravity Sewer

### Longitudinal Profile

- LP 1 LP 2 LP 3 LP 4 LP 5 LP 6 LP 7 LP 8 LP 9 LP 10 LP 11 LP 12 LP 13 LP 14
- LP 14
  LP 15
  LP 16



LLOYDMINSTER



🚍 💂 Study Area

Existing Manhole

#### **Existing Pipe Diameter**

·	100 mm
	150 mm
;	200 mm
	250 mm
:	300 mm
	375 mm
	400 mm
	450 mm
	500 mm
	525 mm
(	600 mm
(	675 mm
·	750 mm
9	900 mm
	1050 mm
	1200 mm
·	1350 mm
	Unknown

#### Proposed Pipe Upgrade Diameter

250 mm 300 mm 375 mm 450 mm 525 mm 600 mm 900 mm





Study Area

# Maximum HGL Elevation Relative To Ground

- Less than -3.5m
- Between -3.5m and -2.4m
- Between -2.4m and 0.0m
- Greater than 0.0m

# Peak Discharge Relative to Pipe Capacity

- Less than 86%
- Between 86% and 100%
- Between 100% and 120%
- Greater than 120%





- Study Area
- Manhole

### Pipe Spare Capacity

- Less than 0L/s
- 0 25L/s
- \_\_\_\_ 25 50L/s
- **50 75L/s**
- 75 100L/s
- Greater than 100L/s





### 6.3 Condition Assessment

The City's Sanitary Sewer Condition Assessment and Management Strategy (AECOM, 2016) identified categories for the City's sanitary sewer infrastructure based on consequence of structural failure of the pipe and associated cost-benefit ratio. These categories were then separated into strategic and non-strategic sub-categories based on the critical operation of the overall sanitary sewer collection system to recommend condition assessment priority areas.

The City provided flooding records for parcels and roads for two (2) significant rainfall events in the city: one in June 2017 and the other in June 2018. The 2016 condition assessment recommendations as well as the two (2) sets of flooding records are shown in Figure 6.12. It should be noted that the A-Strategic category was also separated into sub-categories based on the pipe diameter being less or greater than 600 mm, pipe material not being PVC, and the installation year being before 1990 to aid in the prioritization of proposed condition assessment locations.

The recommended condition assessment areas outlined in the Sanitary Sewer Condition Assessment and Management Strategy (AECOM, 2016) were used in conjunction with the flooding records provided by the City to propose a condition assessment plan as part of this SSMP. This was further refined, with input from the City, to collect condition information for various pipe materials of varying ages to provide an estimate of the condition of the City's overall sanitary sewer collection system. The proposed condition assessment locations are shown in Figure 6.13.

McGill's Industrial Services (McGill's) undertook closed-circuit television (CCTV) inspections of the proposed assessment pipes based on priority. The CCTV reports and videos provided by McGill's are included in Appendix E and Appendix F, respectively.

ISL reviewed the videos and inspection reports, which provided a general condition rating for each section of inspected pipe as well as each overall assessment segment. These ratings are summarized in Table 6.4.

Rating	Condition	Description
1	Excellent	No further action required.
2	Good	Maintenance is recommended. This category was applied to pipe sections with service connection intrusions into the sewer, attached deposits or debris in the pipes, and root intrusions.
3	Fair	Repairs are recommended in the next ten (10) to 20 years. This category was applied to pipe sections with visible ponding due to sags in the pipes or at the joints and pipe surface damage.
4	Poor	Repairs are recommended in the next five (5) to ten (10) years. This rating was applied to pipe sections with visible cracks, breakage, and displacement at joints.
5	Failing	Repairs are recommended immediately. This category was applied to pipe sections that have collapsed or when pipes have produced a hole with a visible void.

#### Table 6.4: Pipe Condition Rating Summary



- Study Area
- Manhole
- ----- Gravity Sewer

## June 2017 Parcel Flooding

- Complaints
- FB Images
- Insurance Claims
- June 10 Sewer Backups
- June 9 Sewer Backups
- June 9 Surface Flooding

## June 2018 Parcel Flooding

- Complaints
- Insurance Claims

## 2016 Condition

- A Strategic (<600mm, Not PVC, installed before 1990) A - Strategic (>600mm, Not
- A Strategic (>600mm, Not PVC, installed before 1990)
- A Strategic (3 km)
- A Non-Strategic (39.6 km)
- B Strategic (1.8 km)
- B Non-Strategic (98.8 km)
- C Non-Strategic (7.4 km)
- N/A





- Study Area
- Manhole
- Gravity Sewer

## 2023 Condition Assessment

- Priority 1
- Priority 2







ISL also compared the pipe age based on the data provided by the City to the expected useful life of the pipe based on material. The expected useful life of pipe materials, based on normal practice, is summarized in Table 6.5.

 Table 6.5:
 Pipe Material Expected Useful Life

Material	Abbreviation	Ideal Expected Useful Life <sup>1</sup>		
Material		Years		
Concrete	CONC	60		
Polyvinyl Chloride	PVC	60		
Steel	STL	50		
Vitrified Clay Tile	VCT	60		

<sup>1</sup> Installation means and the methods as well as pipe application may impact the estimate of expected useful life.

The results of the condition assessment are summarized in Table 6.6 and shown in Figure 6.14.





### Table 6.6: Condition Assessment Summary

Segment No.	Report No.	Start MH	End MH	Surveyed Length	Diameter	Material	Installation Year	Pipe Age	ldeal Remaining Expected	Condition Rating	Condition Rating	Weighted Calculation	Weighted Rating	Segment Condition
				m	mm				Life <sup>1</sup>		Number		Number	Rating
1A	1	2882	2881	99.6	200	PVC	2010	13	47	Good	2	199.2	2.00	Good
1B	2	85	84	100.2	200	VCT	1965	58	2	Failing	5	501	5.00	Failing
2A	12	357	356	68.5	375	CONC	1990	33	27	Fair	3	205.5	1.99	Good
	13	356	2882	70.4	375	CONC	1990	33	27	Excellent	1	70.4		
	34	561	544	58.1	375	VCT	1974	49	11	Poor	4	232.4		
	35	544	655	72.7	375	VCT	1974	49	11	Poor	4	290.8	-	
2B	36	655	543	26.8	375	VCT	1974	49	11	Poor	4	107.2	4 00	Poor
20	37	543	656	41.4	375	VCT	1974	49	11	Poor	4	165.6	4.00	1 001
	38	656	542	58.8	375	VCT	1974	49	11	Poor	4	235.2		
	39	542	461	47.5	375	VCT	1974	49	11	Poor	4	190		
	30	389	388	99.3	375	CONC	1966	57	3	Poor	4	397.2		
2	31	390	389	94.9	375	CONC	1966	57	3	Fair	3	284.7	3.46	Fair
3	32	429	390	21.2	375	CONC	1967	56	4	Fair	3	63.6		
	33	429	434	8.0	375	CMP	1968	55	-5	Poor	4	32	4.00	Poor
	3	528	UNK	41.3	200	VCT	1973	50	10	Good	2	82.6		
4	4	UNK	527	55.6	200	VCT	1973	50	10	Poor	4	222.4	3.58	Poor
	5	527	512	101.7	200	VCT	1973	50	10	Poor	4	406.8		
	8	896	897	56.6	300	VCT	1980	43	17	Good	2	113.2	2.00	
	9	897	996	52.5	300	VCT	1980	43	17	Good	2	105		Good
5	10	996	889	53.4	300	VCT	1980	43	17	Good	2	106.8		
	11	889	857	61.0	300	VCT	1980	43	17	Good	2	122		
	27	691	690	103.1	750	CONC	1980	43	17	Fair	3	309.3		
6	28	690	689	105.6	750	CONC	1980	43	17	Fair	3	316.8	3.00	Fair
	29	691	692	105.0	750	CONC	1980	43	17	Fair	3	315		
	22	1641	1273	88.1	450	PVC	2004	19	41	Excellent	1	88.1		
	23	1273	1642	51.9	450	PVC	2004	19	41	Excellent	1	51.9		
7	24	1642	1644	77.2	450	PVC	2004	19	41	Excellent	1	77.2	1.00	Excellent
	25	1644	1643	53.3	450	PVC	2004	19	41	Excellent	1	53.3		
	26	1643	838	54.6	450	PVC	2004	19	41	Excellent	1	54.6		
	6	1492	842	98.4	300	PVC	2002	21	39	Excellent	1	98.4		
8	7	842	5420	91.0	300	PVC	2002	21	39	Excellent	1	91	1.00	Excellent
	20	1351	1352	89.7	525	PVC	1997	26	34	Excellent	1	89.7		
9	21	1352	1344	91.5	525	PVC	1997	26	34	Excellent	1	91.5	1.00	Excellent
	14	1662	1663	110.6	375	PVC	2005	18	42	Excellent	1	110.6		
10	15	1662	1661	89.5	375	PVC	2005	18	42	Excellent	1	89.5	1.00	Excellent
	15	515	514	66	375		1073	50	10	Poor	1	264		
	17	513	515	19.4	375		1072	50	10	Cood	4 0	204		
11	10	514	510	00.4	275		1072	50	10	Good	2	165.0	3.49	Fair
	10	514	510	02.0	3/3		1973	50	10	Good	<u>ک</u>	100.2	_	
	19	513		77.9	3/5		1973	50	10		<u>р</u>	389.5	2.00	
22	40	613	614	/5.8	200		1976	47	13	Good	2	151.6	2.00	Good

<sup>1</sup> The remaining expected life of the pipe is based on the pipe age and material only with the condition rating recommending the timeline for replacement as needed.



- Study Area
- Manhole
- ----- Gravity Sewer

## **Condition Rating**

- Excellent
- Good
- Fair
- Poor
- Failing







An estimated condition rating was developed for the City's entire sanitary sewer collection system based on the condition assessment results. These results were combined with the pipe installation year and material information from the City to estimate each pipe section's relative condition rating. A summary of the generalized condition assessment results by decade is provided in Table 6.7 with the estimated pipe condition for the existing sanitary sewer collection system shown in Figure 6.15. This can aid the City in identifying future locations for inspections, as well as combining potential replacement opportunities with other upgrades in the area.

Installation Year	Material	Condition Rating <sup>1</sup>
1940-1949	All	Inconclusive
1950-1959	All	Inconclusive
1060 1060	CONC	Fair
1960-1969	VCT	Poor
1970-1979	VCT	Poor
	CONC	Fair
1980-1989	VCT	Good
	PE	Good
1000 1000	CONC	Good
1990-1999	PVC	Excellent
2000-2009	PVC	Excellent
2010-2021	PVC	Excellent

#### Table 6.7: Generalized Condition Assessment Summary by Decade

<sup>1</sup> Any pipes noted as inconclusive did not have a comparative sample pipe segment assessed as part of the 2023 Condition Assessment.

It should be noted that these results are only an estimate based on the sections of pipe that were inspected as part of the condition assessment. Therefore, there may be notable deviations from the generalizations outlined in Table 6.7 when looking at the overall sanitary sewer collection system. For example, the condition rating for the pipe sections that were inspected for the 1980 to 1989 installation years are a potential outlier as it is not expected that VCT pipe is in better condition than CONC pipe from the same decade.

Approximately 20% of the city's existing sanitary sewer collection system was installed in the 1970s with a generalized condition rating of poor as noted in Table 6.7. However, as this is only a generalized condition rating based on the sections of pipe that were included within the condition assessment completed in 2023, it is not recommended that all pipes installed within this timeframe be replaced. Rather, it is recommended that monitoring and/or inspection of pipes with lower generalized condition ratings be undertaken to determine specific conditions and prioritize replacement needs.

Additionally, no pipes installed before 1960 were inspected as part of the condition assessment. Therefore, it is recommended that these pipes be monitored and/or inspected as these pipes are beyond their ideal expected life.



- Study Area
- Manhole

#### 2023 Condition Assessment Rating

- Excellent
- Good
- Fair
- Poor

# Estimated Condition Rating by Installation Year and Material

- Excellent
- Good
- Fair
- Poor
- Inconclusive

Any pipes noted as inconclusive did not have a comparative sample pipe segment assessed as part of the 2023 Condition Assessment.







### 6.3.1 Existing System Condition Recommendations

Based on the existing sanitary sewer collection system condition assessment, upgrades were developed to improve areas of concern where the pipe condition rating was considered either fair, poor or failing. The identified upgrades are summarized in Table 6.8 and shown in Figure 6.16 with the segment numbers identified in Figure 6.13 and the report numbers identified in Figure 6.14.

Upgrade No.	Name	Description	Condition Rating	Segment No.	Report No.
14	49 Avenue Sewer Replacement	Replacement of 200 mm sewer along 49 Avenue from 38 Street to 39 Street.	Failing	1B	2
15	42 Street East Sewer Replacement	Replacement of 375 mm sewer along 42 Street from west of 49 Avenue to 50 Avenue.	Fair	2A	12
16	42 Street West Sewer Replacement	Replacement of 375 mm sewer along 42 Street from 52 Avenue to 54 Avenue.	Poor	2B	34-39
17	36 Street Sewer Replacement	Replacement of 375 mm sewer along 36 Street from 48 Avenue to 50 Avenue.	Poor/Fair	3	30-33
18	52 Avenue Sewer Replacement	Replacement of 200 mm sewer along 52 Avenue from 34 Street to 35 Street.	Poor	4	4-5
19	46 Avenue Sewer Replacement	Replacement of 750 mm sewer along 46 Avenue from 25 Street to 27 Street.	Fair	6	27-29
20	35 Street Sewer Replacement	Replacement of 375 mm sewer along 35 Street from 52 Avenue to 54 Avenue.	Poor/Failing	11	16-19

Table 6.8:	Existing	System	Proposed	Condition	Upgrades
------------	----------	--------	----------	-----------	----------



- Study Area
- Existing Manhole

## **Existing Pipe Diameter**

- 100 mm – 150 mm 200 mm 250 mm 300 mm - 375 mm – 400 mm 450 mm 500 mm 525 mm 600 mm – 675 mm 750 mm – 900 mm — 1050 mm 1200 mm 1350 mm
- ----- Unknown

### Proposed Pipe Replacement

- \_\_\_\_ 200 mm
- **——** 375 mm
- **—** 750 mm







### 6.4 Risk Assessment

ISL prepared a risk assessment of the recommended upgrades to aid the City in prioritizing potential infrastructure projects. This risk assessment was based on the following criteria, which is described in detail in Table 6.9:

- Average Change in HGL;
- HGL Impacts;
- Existing Upstream Impact;
- Future Upstream Impact;
- Generalized Pipe Condition; and
- Road Condition Upgrade Potential.

#### Table 6.9: Existing System Upgrades Risk Assessment – Risk Criteria and Scoring

			Scoring	
ID	Name	Definition	Scale	Description
			5	Significant
		Average change in HGL between existing	4	
C.1	Average Change in HGL	conditions and proposed upgrade conditions	3	Moderate
		based on model results	2	
			1	Minimal
			5	Significant
		Number of basements and populations		
C.2	HGL Impact (2.4 m)	connected to a manhole surcharging to basement level within 2.4 m of the surface	3	Moderate
		based on model results	2	
			1	Minimal
				Significant
		Number of basements and populations	4	
C.3	HGL Impact (3.0 m)	connected to a manhole surcharging to basement level within 3.0 m of the surface	3	Moderate
	()	based on model results	2	
			1	Minimal
			5	Significant
		Number of basements and populations	4	
C.4	HGL Impact (3.5 m)	t connected to a manhole surcharging to		Moderate
	(,	based on model results	2	
				Minimal





			Scoring	
			5	Significant
		Number of basements and populations	4	
C.5	HGL Impact (4 0 m)	connected to a manhole surcharging to basement level within 4.0 m of the surface	3	Moderate
	(4.0 m)	based on model results	2	
			1	Minimal
			5	Significant
		Number of parcels and catchment area	4	
C.6	Existing Upstream Impact	upstream of upgrade under existing	3	Moderate
	oporroan impaor	conditions		
			1	Minimal
	C.7 Future Upstream		5	Significant
			4	
C.7		Number of parcels and catchment area	3	Moderate
	impuot	approach of appraide and of ratio of official	2	
			1	Minimal
			5	Failing
		General condition of existing nine based on	4	Poor
C.8	Generalized Pipe	condition assessment and generalized pipe	3	Fair
	Condition	condition rating	2	Good
			1	Excellent
			5	Excellent
	Road Condition	Potential for upgrades to be coupled with	4	Good
C.9	Upgrade	likely to incorporate road improvements	3	Fair
	Potential	based on road condition from Google Street View	2	Poor
			1	Negligible

A pairwise comparison was used to allocate a weighting to each criterion as shown in Table 6.10.



## Table 6.10: Existing System Upgrades Risk Assessment – Criteria Ranking

Risk Criteria Pairwise Comparison								Count	Maighting		Cri	iteria Ranking		
	C.1	C.2	C.3	C.4	C.5	C.6	C.7	C.8	C.9	Count	weighting	Rank	ID	Description
C.1	C.1	C.1	C.1	C.1	C.1	C.1	C.1	C.1	C.1	9	20.0%	1	C.1	Average Change in HGL
C.2		C.2	C.2	8	17.8%	2	C.2	HGL Impact (2.4m)						
C.3			C.3	C.3	C.3	C.3	C.3	C.3	C.3	7	15.6%	3	C.3	HGL Impact (3.0m)
C.4				C.4	C.4	C.4	C.4	C.4	C.4	6	13.3%	4	C.4	HGL Impact (3.5m)
C.5					C.5	C.5	C.5	C.5	C.5	5	11.1%	5	C.5	HGL Impact (4.0m)
C.6						C.6	C.6	C.6	C.6	4	8.9%	6	C.6	Existing Upstream Impact
C.7							C.7	C.7	C.7	3	6.7%	7	C.7	Future Upstream Impact
C.8								C.8	C.8	2	4.4%	8	C.8	Generalized Pipe Condition
C.9									C.9	1	2.2%	9	C.9	Road Condition Upgrade Potential
Total									45	100%				

	-	_	





A summary of the prioritization results of the risk assessment is provided in Table 6.11 with the risk assessment details and calculations provided in Appendix G.

Priority	Upgrade No.	Name	Category	Length (m)	Overall Score
1	6	36 Street Trunk Upgrade - West	Capacity	402.17	2.64
2	9	Southeast Trunk Upgrade	Capacity	833.93	2.38
3	4	52 Street Trunk Upgrade	Capacity	807.62	1.76
4	3	51 Street Sewer Upgrade	Capacity	916.34	2.31
5	10	47 Avenue Sewer Upgrade	Capacity	234.14	2.24
6	2	44 Street Sewer Upgrade	Capacity	273.47	1.71
7	5	48 Avenue Sewer Upgrade	Capacity	297.77	1.69
8	7	36 Street Trunk Upgrade - East	Capacity	851.11	1.47
9	8	East Trunk Upgrade	Capacity	1060.35	1.42
10	12	25 Street Trunk Upgrade	Capacity	386.00	1.38
11	11	53 Avenue Sewer Upgrade	Capacity	487.69	1.36
12	13	59 Avenue Sewer Upgrade	Capacity	502.16	1.13
13	19	46 Avenue Sewer Replacement	Condition	315.60	0.89
14	17	36 Street Sewer Replacement	Condition	156.80	0.47
15	14	49 Avenue Sewer Replacement	Condition	100.93	0.36
16	18	52 Avenue Sewer Replacement	Condition	148.15	0.36
17	20	35 Street Sewer Replacement	Condition	164.92	0.36
18	16	42 Street West Sewer Replacement	Condition	358.06	0.33
19	15	42 Street East Sewer Replacement	Condition	71.02	0.31

 Table 6.11: Existing System Upgrades Risk Assessment Priority Summary

As previously discussed, the Aurora Sewer Upgrades (Upgrade 1) are not included in the prioritization due to a lack of operational issues to date. It is recommended that this location be monitored in the future and added to the prioritization if required.

Despite the lower overall score, the 52 Street Trunk Upgrade (Upgrade 4) has been placed above the 51 Street Sewer Upgrade (Upgrade 3) due to the constructability of the improvements. Completing Upgrade 4 prior to Upgrade 3 ensures that downstream infrastructure is upgraded prior to the infrastructure upstream of it.





## 6.5 Existing Sanitary System Upgrades Cost Estimates

A summary of the costs associated with the recommended existing sanitary sewer collection system upgrades are detailed below in Table 6.12. The list is ordered based on upgrade IDs, while priorities are included for reference. A full breakdown of the costs has been provided in Appendix H.

Category	Upgrade No.	Priority	Total Cost <sup>1</sup>
	1	N/A	\$1,020,000
	2	6	\$700,000
	3	4	\$1,410,000
	4	3	\$2,530,000
	5	7	\$4,990,000
	6	1	\$1,220,000
Capacity	7	8	\$2,950,000
	8	9	\$4,110,000
	9	2	\$2,570,000
	10	5	\$630,000
	11	11	\$1,350,000
	12	10	\$1,110,000
	13	12	\$1,920,000
	Сарас	city Upgrades Sub-Total	\$26,510,000
	14	15	\$270,000
	15	19	\$220,000
	16	18	\$1,040,000
Condition	17	14	\$470,000
	18	16	\$370,000
	19	13	\$1,220,000
	20	17	\$480,000
	\$4,070,000		
	\$30,580,000		

Table 6.12: Cost Estimates for Recommended Existing System Upgrades

<sup>1</sup>The total cost has been rounded to the nearest \$10,000 and includes a 30% contingency as well as 15% for engineering.

As previously discussed, the Aurora Sewer Upgrades (Upgrade 1) is not included in the prioritization due to a lack of operational issues to date; the cost estimate has been included for the City's reference in the overall picture. It is recommended that this location be monitored in the future and added to the prioritization if required.



# **7.0** Future Sanitary System

## 7.1 Future System Concept Development

The future network assumes that all the recommended existing sanitary sewer collection system upgrades are implemented. Thus, it is recommended that these upgrades are completed prior to any substantial densification or future development. A spreadsheet approach for infrastructure sizing was adopted, based on the DWF residential and industrial, commercial, institutional (ICI) generation rates, peaking factors, and the I-I allowance rate outlined in Section 3.3.

The specified pipe diameters are the smallest possible determined based on the minimum design slope to provide a self-cleansing full-sewer velocity of greater than 0.60 m/s as presented in Table 7.1. This was calculated under the derived peak WWFs, based on a roughness coefficient of 0.013.

Nominal Sewer Size	Minimum Design Slope		Full-Sewer Velocity	Full-Sewer Capacity
mm	%	m/m	m/s	L/s
200	0.40	0.0040	0.66	20.7
250	0.28	0.0028	0.64	31.5
300	0.22	0.0022	0.64	45.4
375	0.15	0.0015	0.62	67.9
450	0.12	0.0012	0.62	98.8
525	0.10	0.0010	0.63	136.0
600 (and larger)	0.10	0.0010	0.69 <sup>1</sup>	194.2 <sup>1</sup>

#### Table 7.1: Minimum Design Slopes for Sewers

<sup>1</sup> Reflects a sewer diameter of 600 mm, noting that anything larger will have increased full-sewer velocities and capacities.

If flatter slopes are preferred or required at the detailed design stages, this can be reviewed on a case-bycase basis, though it would have negative repercussions (i.e., reduced capacities). If this was acceptable, the determined pipe diameters would need to be increased to meet the specified flow designs. Contrarily, steeper slopes could potentially be achieved depending on topography and how the developments are ultimately graded. This could result in a potentially smaller pipe diameter, which again, should be reviewed during detailed design.

With regards to pumped flows, a new forcemain is typically designed to operate at a preferred velocity range of 0.76 m/s to 1.5 m/s. This approach was used to size new forcemains for the purpose of developing a future servicing concept that minimizes head losses, in turn yielding savings on the energy consumption front. Ensuring that forcemains are sized for a minimum velocity of 0.76 m/s helps to keep materials suspended, thus decreasing sediment build-up in the sewer. Each future forcemain was specified as a single pipe. To provide a degree of redundancy, as well as staging of flows, twinned pipes with an equivalent capacity could be considered.

The servicing schemes of the proposed sanitary sewer collection system are conceptual. Ultimately, it will be up to the developer to fulfill the intent of the servicing concept presented herein. Therefore, a developer may choose to adjust the alignment of the specified trunks as needed, to accommodate the sanitary sewer collection system within future developments. A developer may also choose to connect services directly to the future sanitary trunks if found beneficial, provided the designed sanitary sewer collection system does not result in any negative impacts on the directly connected developments.





Specifically, surcharge conditions within the sanitary sewer collection system resulting in basement backups is of concern.

Figures 7.1 and 7.2 show the proposed sanitary sewer collection system under the Ultimate Boundary Horizon (2051) for two (2) concepts, respectively. Both figures illustrate the proposed sanitary servicing concept and pipe diameter, direction of flow to each minimum parcel elevation along with the minimum parcel elevations, and locations and manhole IDs of any ties to the existing sanitary sewer collection system.

One lift station and forcemain are proposed in the north to service approximately half a quarter section adjacent to the Canadian Pacific Railway (CPR). There is also one area north of the airport that is proposed to remain disconnected from the City's sanitary sewer collection system due to the distance. Septic tanks are recommended for properties in this quarter section; otherwise, a second lift station and forcemain will be needed.

One area is flagged as having a maximum cover depth of 8 to 9 m based on current surface data. This is at the upper limit of the City's tolerance for gravity sewers before a lift station is triggered and should be closely monitored as this section of trunk sewer is built. Re-grading 12 Street to reduce the cover, though likely much more costly upfront, could have longer term savings with reduced operational and maintenance costs. This could be considered as an alternate solution. The City could also consider revising the alignment slightly to an easement within the development, if there is the potential to re-grade the easement with the intent of reducing cover.

Servicing through The Willows is intended to follow the proposed alignment of the South Trunk in The Willows ASP (Select Engineering Consultants Ltd., 2016). The alignment directly downstream through Wallacefield is approximate, based on the Wallacefield Concept Plan (Musgrave Agencies Ltd., et.al., 2014). Overflow from Wallacefield due to pipe constraints, as discussed in Section 6.2.1, are considered in both concepts. The South Trunk ultimately ties into MH 640 in the Aurora neighbourhood, where upgrades to the existing sanitary sewer collection system have already been completed in anticipation of this trunk. A new south-to-north trunk (the East Trunk) then conveys flows ultimately to the WWTP.

The Willows ASP indicates a proposed sanitary servicing pipe that terminates just east of 50 Avenue between two (2) proposed commercial parcels. The servicing concept recommended in this SSMP has the quarter section west of 50 Avenue and south of 12 Street being routed through a 300 mm sewer north on 50 Avenue and east on 12 Street. Depending on the ultimate size of the pipe that terminates east of 50 Avenue, there is the potential to route the quarter section through The Willows rather than around. This could reduce the overall cost of the servicing concept and avoid a major construction project along two (2) major roadways.

The notable differences between the two (2) concepts are as follows:

- **Concept 1**: peak WWF from the single-family residential quarter section adjacent to Parkview Estates ties into the existing sanitary sewer collection system at tie-in point #660 shown in Figure 7.1. This could be an ideal solution if this development progresses at a quicker pace than the parcels south of 12 Street; however, this concept relies more on the capacity of the existing infrastructure.
- **Concept 2**: peak WWF from the single-family residential quarter section adjacent to Parkview Estates is routed south and ties into the servicing concept proposed for the parcels south of 12 Street as shown in Figure 7.2. Though this concept may require fewer upgrades to the existing sanitary sewer collection system, it is reliant on installation of proposed infrastructure to the south (South Trunk), thus making timelines more challenging to manage.



- Gravity Tie-In Location \* Servicing Area Minimum Elevation  $\mathbf{\bullet}$ Proposed Lift Station ☆ Wastewater Treatment Plant ----- Existing Sewer Study Area Future Parcel Tie-In New Service Existing Service Lift Station Required Septic Tanks Serviced But Vacant Airport Expansion **Proposed Pipe Diameter** 200 mm Forcemain → 300 mm Sewer → 375 mm Sewer ▶ 450 mm Sewer ▶ 525 mm Sewer ▶ 600 mm Sewer 675 mm Sewer ▶ 750 mm Sewer
  - ➡ 900 mm Sewer
  - 1050 mm Sewer
  - 1200 mm Sewer

Service area minimum points (green) and tie-in elevations at the upstream invert of the existing pipe (blue) are in m.

Maximum cover depths provided are approximate ranges.

Sizing assumes single forcemains. Twinned forcemains can be considered for optimal redundancy if an equivalent capacity is accomplished.





- Wastewater Treatment Plant
- ----- Existing Sewer
- Study Area

#### Future Parcel Tie-In

New Service Existing Service Lift Station Required Septic Tanks Serviced But Vacant Airport Expansion

#### Proposed Pipe Diameter

200 mm Forcemain
 300 mm Sewer
 375 mm Sewer
 450 mm Sewer
 525 mm Sewer
 600 mm Sewer
 675 mm Sewer
 750 mm Sewer
 900 mm Sewer
 1050 mm Sewer
 1200 mm Sewer
 1350 mm Sewer

Service area minimum points (green) and tie-in elevations at the upstream invert of the existing pipe (blue) are in m.

Maximum cover depths provided are approximate ranges.

Sizing assumes single forcemains. Twinned forcemains can be considered for optimal redundancy if an equivalent capacity is accomplished.



FIGURE 7.2 FUTURE SERVICING CONCEPT 2 LLOYDMINSTER SANITARY SEWER MASTER PLAN









## 7.2 Future System Concept Cost Estimates

The cost estimate summary for both servicing concepts are summarized below in Table 7.2. For a detailed cost breakdown, please refer to Appendix H. The costs are stipulated for the infrastructure necessary for the proposed concept only. An assessment of the servicing concept and required upgrades follows. The items included in Table 7.2 are only for new infrastructure required for the two (2) servicing concepts, and do not include any upgrades to the existing sanitary sewer collection system.

Category	Item	Total Cost <sup>1</sup>
	200 mm Forcemain	\$740,000
	300 mm Gravity Sewer	\$870,000
	375 mm Gravity Sewer	\$1,560,000
	450 mm Gravity Sewer	\$5,220,000
	525 mm Gravity Sewer	\$1,160,000
Concept 1	600 mm Gravity Sewer	\$4,450,000
Concept 1	675 mm Gravity Sewer	\$3,280,000
	750 mm Gravity Sewer	\$3,480,000
	900 mm Gravity Sewer	\$3,110,000
	1050 mm Gravity Sewer	\$4,350,000
	1200 mm Gravity Sewer	\$26,720,000
	Lift Station 1 (31 L/s)	\$1,050,000
	Future Concept 1 Total	\$55,990,000
	200 mm Forcemain	\$740,000
	300 mm Gravity Sewer	\$870,000
	375 mm Gravity Sewer	\$1,560,000
	450 mm Gravity Sewer	\$5,570,000
	525 mm Gravity Sewer	\$1,160,000
	600 mm Gravity Sewer	\$5,250,000
Concept 2	675 mm Gravity Sewer	\$1,380,000
	750 mm Gravity Sewer	\$4,080,000
	900 mm Gravity Sewer	\$3,010,000
	1050 mm Gravity Sewer	\$2,790,000
	1200 mm Gravity Sewer	\$25,580,000
	1350 mm Gravity Sewer	\$6,280,000
	Lift Station 1 (31 L/s)	\$1,050,000
	\$59,320,000	

Table 7.2:	Cost Estimates for	Recommended Future	Sanitary Servicing Concepts
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<sup>1</sup>The total cost has been rounded to the nearest \$10,000 and includes a 30% contingency as well as 15% for engineering.







## 7.3 Future System Assessment

The future sanitary sewer collection system was assessed for the LOS rainfall event (i.e., the 1:25 year storm) for both concepts to determine what, if any, upgrades to the existing sanitary sewer collection are required. Both concepts were assessed under the Ultimate Boundary, while the preferred option was carried forward for staging under the interim growth horizons discussed in Section 2.3 as follows, noting that all development in the 3-Year Growth Horizon ties to existing:

- 3-Year Growth (2025) Population of 34,651;
- 5-Year Growth (2027) Population of 36,132;
- 10-Year Growth (2032) Population of 41,148;
- 20-Year Growth (2042) Population of 57,271; and
- Ultimate Boundary (2051) Population of 67,149.

#### 7.3.1 Concept Assessments and Recommended Upgrades to Existing Infrastructure

The performance of the existing sanitary sewer collection system under the build-out to the Ultimate Boundary Horizon was assessed using the criteria discussed in Section 3.2. The calibrated existing sanitary sewer collection system model was run under the 1:25 year 24-hour Huff design storm under this horizon. The purpose of this assessment was to ensure that the proposed servicing concept is effective and that the trunks that tie-into the existing sanitary sewer collection system do not undermine the downstream pipes. The results of this analysis are shown in Figures 7.3 to 7.6 for Concepts 1 and 2, illustrating the maximum HGL and peak discharge relative to pipe capacity and spare capacity, respectively. The results are summarized for both concepts below. Future sanitary sewer collection system assessment longitudinal profiles for both concepts are shown in Appendix D.

Regarding the existing sanitary sewer collection system, there were a total of seven (7) notable locations between the two (2) concepts (many overlapping) where the additional tie-ins caused surcharging in the downstream pipes. These locations, along with a description of any upgrades that would be required to alleviate any constraints, are shown in Table 7.3.



ID	LP No.	Concept Triggered	Description	Upgrade?
1	1	1 and 2	Very minor surcharging with some sections of pipe illustrating an HGL above the crown of the pipe. This is a flatter and downsized section of pipe, thus the reduced capacity. Recommend replacement to a 1,200 mm with next roadworks in the vicinity.	With Roadworks
2	2	1 and 2	Apparent surcharging, HGL above crown of the pipe therefore upgrades triggered. Recommend replacement with a 675 mm sewer.	Yes
3	3	1 and 2	Very minor surcharging, HGL remains within the pipe; therefore, no upgrades triggered.	No
4	4	1 and 2	Very minor surcharging, HGL remains within the pipe; therefore, no upgrades triggered.	No
5	5	1	Some surcharging in localized section of pipe. Recommend replacement with a 300 mm sewer.	Yes
6	6/7	1	Very minor surcharging, HGL remains within the pipe; therefore, no upgrades triggered.	No
7	6/7	1	Very minor surcharging, HGL remains within the pipe; therefore, no upgrades triggered.	No

#### Table 7.3: Areas of Concern and Recommended Upgrades

The upgrades recommended in this SSMP consider pipe upsizing along the same alignment as current deficiencies. Alternatives may include pipe twinning, new pipe alignments, or offloading to existing sanitary sewers with spare capacity. The City may wish to consider these alternatives to pipe upsizing during preliminary design, as long as the ultimate intent of the upgrades are achieved.

The three (3) upgrades (full upgrades or during roadworks, aka Upgrades 1, 2, and 5) described in Table 7.3 are illustrated in Figure 7.7. Assessment results under the 1:25 year 24-Hour Huff design storm are shown in Figures 7.8 to 7.11 for maximum HGL relative to surface and peak discharge relative to pipe capacity and spare capacity, respectively, for both concepts. Results indicate that surcharging is alleviated in the sewers that were recommended for upgrades. Longitudinal profiles of these three (3) upgrade locations are included in Appendix D.

The 600 mm sewer upstream of Upgrade 2, between 59 Avenue and 63 Avenue, shows some minor surcharging after the upgrades are implemented. The HGLs for both concepts are 2.5 m below the ground and were not flagged for upgrades. Based on the City's risk tolerance, this section could potentially be upsized to a 675 mm sewer to completely mitigate surcharging. It is recommended that this section of sewer is monitored as development proceeds in the west to monitor its performance. Upgrades could be completed if the capacity and condition of the sewer worsens and exceeds the assessment criteria of maintaining a freeboard of at least 2.5 m to the surface.



- ☆ Wastewater Treatment Plant
- Proposed Lift Station
- → Proposed Pipe
- Proposed Forcemain
- Study Area

# Maximum HGL Elevation Relative To Ground

- Less than -3.5m
- Between -3.5m and -2.4m
- Between -2.4m and 0.0m

#### Peak Discharge Relative To Pipe Capacity

- Less than 86%
- Between 86% and 100%
- Greater than 100%





- ★ Wastewater Treatment Plant
- Proposed Lift Station
- → Proposed Pipe
- Proposed Forcemain
- Study Area

## Spare Capacity

- Less than 0L/s
- 0 25L/s
- \_\_\_\_ 25 50L/s
- 50 75L/s
- 75 100L/s
- Greater than 100L/s





- ★ Wastewater Treatment Plant
- Proposed Lift Station
- → Proposed Pipe
- Proposed Forcemain
- Study Area

# Maximum HGL Elevation Relative To Ground

- Less than -3.5m
- Between -3.5m and -2.4m
- Between -2.4m and 0.0m

# Peak Discharge Relative To Pipe Capacity

- Less than 86%
- Between 86% and 100%
- Greater than 100%







- Manhole
- ★ Wastewater Treatment Plant
- Proposed Lift Station
- Proposed Pipe
- Proposed Forcemain
- Study Area

## Spare Capacity

- —— Less than 0L/s
- 0 25L/s
- 25 50L/s
- 50 75L/s
- **—** 75 100L/s
- Greater than 100L/s







★ Wastewater Treatment Plant

Study Area

Existing Manhole

## Existing Pipe Diameter

 100 mm
 150 mm
 200 mm
 250 mm
 300 mm
 375 mm
 400 mm
 450 mm
 500 mm
 525 mm
 600 mm
 675 mm
 750 mm
 900 mm
 1050 mm
 1200 mm
1350 mm
 Unknown

Private

## Proposed Future Pipe Upgrade Diameter

300	mm
 675	mm

— 1200 mm









- 🔆 Wastewater Treatment Plant
- Proposed Lift Station
- ----> Proposed Pipe
- Proposed Forcemain
- Study Area

# Maximum HGL Elevation Relative To Ground

- Less than -3.5m
- Between -3.5m and -2.4m
- Between -2.4m and 0.0m

# Peak Discharge Relative To Pipe Capacity

- Less than 86%
- Between 86% and 100%
- Greater than 100%





Plan/251 Figures/3 Reporting/Figure 7.9 Future Servicing Concept 1 Upgrades 25-yr 24-hr Huff Results - Spare Capacity.mv

## Legend

- Manhole
- ★ Wastewater Treatment Plant
- Proposed Lift Station
- Proposed Pipe
- Proposed Forcemain
- Study Area

## Spare Capacity

- Less than 0L/s
- 0 25L/s
- 25 50L/s
- 50 75L/s
- **—** 75 100L/s
- Greater than 100L/s





- 🔆 Wastewater Treatment Plant
- Proposed Lift Station
- ----> Proposed Pipe
- Proposed Forcemain
- Study Area

# Maximum HGL Elevation Relative To Ground

- Less than -3.5m
- Between -3.5m and -2.4m
- Between -2.4m and 0.0m

# Peak Discharge Relative To Pipe Capacity

- Less than 86%
- Between 86% and 100%
- Greater than 100%





Date: 2024-05-30 Document: Q:\Projects\28114\_Lloyd\_Sanitary\_Master\_Plan\251\_Figures\3\_Reporting\Figure 7.11\_Future Servicing Concept 2 Upgrades\_25-yr 24-hr Huff Results - Spare Capacity.mxd

## Legend

- Manhole
- ★ Wastewater Treatment Plant
- Proposed Lift Station
- Proposed Pipe
- Proposed Forcemain
- Study Area

## Spare Capacity

- Less than 0L/s
- 0 25L/s
- \_\_\_\_ 25 50L/s
- 50 75L/s
- **—** 75 100L/s
- Greater than 100L/s







## 7.4 Existing System Upgrades under Growth Conditions Cost Estimates

Cost estimates are provided in Table 7.4 for both servicing concepts, while detailed breakdowns of the cost estimates are provided in Appendix H.

Category	Upgrade No.	ltem	Total Cost
		900 mm Gravity Sewer Removal	\$250,000
	1	1,200 mm Gravity Sewer	\$2,700,000
		Pavement Rehabilitation	\$1,160,000
		Upgrade 1 Sub-Total	\$4,110,000
	2	600 mm Gravity Sewer Removal	\$190,000
Concept 1	2	675 mm Gravity Sewer	\$1,670,000
		Upgrade 2 Sub-Total	\$1,860,000
		200 mm Gravity Sewer Removal	\$20,000
	6	300 mm Gravity Sewer	\$110,000
		Pavement Rehabilitation	\$180,000
		Upgrade 3 Sub-Total	\$310,000
		Future Upgrades Concept 1 Total	\$6,280,000
	1	900 mm Gravity Sewer Removal	\$250,000
		1,200 mm Gravity Sewer	\$2,700,000
		Pavement Rehabilitation	\$1,160,000
Concept 2		Upgrade 1 Sub-Total	\$4,110,000
	2	600 mm Gravity Sewer Removal	\$190,000
		675 mm Gravity Sewer	\$1,670,000
		Upgrade 2 Sub-Total	\$1,860,000
Future Upgrades Concept 2 Total			\$5,970,000

Table 7.4: Cost Estimates for Recommended Upgrades under Growth Conditions

The estimated cost of extending the Upgrade 2 recommendation of upsizing to a 675 mm sewer west, between 59 Avenue and 63 Avenue is \$1,840,000 (note this includes pavement rehabilitation costs whereas the costs in Upgrade 2 above do not, as no roadways are impacted by development). This is provided for reference only if monitoring indicates that capacity significantly worsens with development, to the extent that the maximum HGL relative to the surface no longer meets the 2.5 m criteria.

As the cost estimates do not vary significantly between the two (2) concepts, it is recommended that the City proceeds with Concept 1. Though some pipes do show elevated HGLs downstream of the additional tie-in point to the existing sanitary sewer collection system, excessive surcharging is generally limited to one additional section of pipe. Concept 1 would allow for greater flexibility of the affected quarter section, as it does not rely on implementation of the South Trunk infrastructure.


#### 7.5 Staging of Preferred Servicing Concept

Staging of the preferred servicing concept (Concept 1) for the 5-, 10-, and 20-Year Growth Horizons are shown in Figures 7.12 to 7.14, respectively. The concepts assume that overflow from Wallacefield (additional 60 L/s) is triggered in the 5-Year Growth Horizon.

The proposed servicing concept staging has been oversized to account for future development up to the Ultimate Boundary (2051) based on an approximate sewer service life of 60 years. Oversizing sewer upgrades for future scenarios is seen as more cost-effective since the difference in pipe cost will be marginal compared to the costs of excavating and rehabilitating roadways multiple times. Oversizing existing sanitary sewer collection system upgrades to account for ultimate development may introduce operational and maintenance concerns, such as odour and sedimentation within upgraded sewer sections due to low velocities. It is noting that detailed design of sewer upgrades may wish to consider additional sewer flushing programs or alternative upgrading concepts, such as staged infrastructure, twinned sewers, or interim smaller diameter sewers housed within the ultimate sewer.

The new 1,200 mm East Trunk sewer is triggered when the spare capacity in the existing 900 mm East Trunk is exhausted, at a limiting spare capacity of approximately 125 L/s under the LOS event. This capacity is exceeded prior to the 10-Year Growth Horizon, suggesting the East Trunk is required within the next ten (10) years. It is recommended that the timing for the East Trunk is revisited in 5-years to confirm the exact timing of this upgrade.



- ☆ Wastewater Treatment Plant
- Existing Sewer
- Study Area

### **Future Parcel Tie-In**



New Service Existing Service Lift Station Required Septic Tanks Serviced But Vacant

Airport Expansion

## **Proposed Pipe Diameter**

- ➡ 300 mm Sewer
- 450 mm Sewer
- 600 mm Sewer
- ➡ 675 mm Sewer
- 1200 mm Sewer

Assumes Ultimate Boundary pipe sizes to avoid multiple construction projects.



FIGURE 7.12 FUTURE SERVICING CONCEPT 1 5 YEAR STAGING PLAN LLOYDMINSTER SANITARY MASTER PLAN







- ★ Wastewater Treatment Plant
- Existing Sewer
- Study Area

## **Future Parcel Tie-In**

- - New Service Existing Service Lift Station Required Septic Tanks Serviced But Vacant
  - Airport Expansion

# **Proposed Pipe Diameter**

- ➡ 300 mm Sewer ➡ 375 mm Sewer 450 mm Sewer ➡ 600 mm Sewer ➡ 675 mm Sewer
  - 1200 mm Sewer

Assumes Ultimate Boundary pipe sizes to avoid multiple construction projects.





- ★ Wastewater Treatment Plant
- Existing Sewer
- Study Area

### **Future Parcel Tie-In**

- - New Service Existing Service Lift Station Required Septic Tanks Serviced But Vacant
  - Airport Expansion

## **Proposed Pipe Diameter**

➡ 300 mm Sewer → 375 mm Sewer 450 mm Sewer 525 mm Sewer ▶ 600 mm Sewer 675 mm Sewer 750 mm Sewer ➡ 900 mm Sewer 1050 mm Sewer 1200 mm Sewer

Assumes Ultimate Boundary pipe sizes to avoid multiple construction projects.







# 8.0 Capital Plan

A Capital Plan staged to the Ultimate Boundary has been developed from the recommendations made by this SSMP. An overview of the capital plan, including all upgrades required to the sanitary sewer collection system by full build-out, the servicing scheme proposed, and the servicing type are illustrated in Figure 8.1 (note that this is reflective of the recommended servicing Concept 1). The following should be considered when reviewing this information:

- A 2.0% inflation increase per year should be considered to the base costs;
- The annual capital budget allowance is meant to hold funding each year for maintenance and "one off" instances where repairs are required;
- The horizon in which the upgrade is suggested is based on discussion between the City and ISL on when development could occur based on a full build-out scenario; and
- High level cost estimates provided are a Class 4 with an accuracy of +75% to -40%.

To provide interim measures to ensure that areas of the city are growth ready, the following staging plan is recommended to align with the capital plan:

- Complete the capacity upgrades recommended to the existing sanitary sewer collection system based on the priority noted in Table 6.12.
- Complete the condition upgrades recommended to the existing sanitary sewer collection system based on the priority noted in Table 6.12. It is noted that upgrades to this infrastructure could also be completed in conjunction with the City's roadworks program to minimize costs.
- Continue with the condition assessment program to identify additional sewers in poor or failing condition.
- Conduct periodic flow monitoring programs to assess the degree of I-I in certain neighbourhoods. Consider pinpointing smaller catchments that are likely to be more susceptible to I-I.
- Progress the future servicing concept as development proceeds, prioritizing infrastructure required to service development in the short-term.
- Monitor the three (3) areas flagged for upgrades due to future development, and target upgrades when the existing spare capacity is exceeded due to additional development. As a corollary, consider the repercussions of I-I and the potential for increased spare capacity if I-I mitigation measures are implemented.

This generalized staging plan is shown in relation to triggered growth horizon in Figure 8.2 and in Table 8.1 below. Cost breakdowns per growth horizon are included in Appendix H. These recommended upgrades are being provided as a staging plan with the intent that the SSMP will be integrated into an overall capital plan and budget. These upgrades are meant to align with the City's roadworks program and would be implemented in conjunction with the road upgrades to reduce capital costs.





#### Table 8.1:Capital Planning Horizons

ID <sup>1</sup>	Туре	Description	Cost	
		3 Years (2024 to 2027)		
EX UPG 1	Existing Upgrade	Removal of 200 mm gravity sewer and installation of 250 mm and 300 mm gravity sewer, along with pavement rehabilitation.	\$1,020,000	
EX UPG 2	Existing Upgrade	Removal of 200 mm gravity sewer and installation of 250 mm gravity sewer, along with pavement rehabilitation along Yellowhead Highway.	\$700,000	
EX UPG 3	Existing Upgrade	Removal of 300 mm gravity sewer and installation of 375 mm gravity sewer, along with pavement rehabilitation and railway crossing.	\$1,410,000	
EX UPG 4	Existing Upgrade	Removal of 450 mm gravity sewer and installation of 300 mm and 525 mm gravity sewer, along with pavement rehabilitation along 52 Street.	\$2,530,000	
EX UPG 5	Existing Upgrade	Removal of 300 mm and 375 mm gravity sewer and installation of 375 mm gravity sewer, along with pavement rehabilitation and railway crossing along 48 Avenue.	\$4,990,000	
EX UPG 6	Existing Upgrade	Removal of 375 mm gravity sewer and installation of 450 mm gravity sewer, along with pavement rehabilitation along 36 Street.	\$1,220,000	
EX UPG 7	Existing Upgrade	Removal of 750 mm gravity sewer and installation of 900 mm gravity sewer, along with pavement rehabilitation along 36 Street.	\$2,950,000	
EX UPG 8	Existing Upgrade	Removal of 750 mm gravity sewer and installation of 900 mm gravity sewer, along with pavement rehabilitation along 41 Street.	\$4,110,000	
EX UPG 9	Existing Upgrade	Removal of 450 mm and 375 mm gravity sewer and installation of 525 mm gravity sewer, along with pavement rehabilitation along 46 Avenue, 47 Avenue, and 19 Street.	\$2,570,000	
EX UPG 10	Existing Upgrade	Removal of 250 mm gravity sewer and installation of 300 mm gravity sewer, along with pavement rehabilitation along 47 Avenue.	\$630,000	
EX UPG 11	Existing Upgrade	Removal of 250 mm and 300 mm gravity sewer and installation of 300 mm and 375 mm gravity sewer, along with pavement rehabilitation along 53 Avenue.	\$1,350,000	
EX UPG 12	Existing Upgrade	Removal of 300 mm gravity sewer and installation of 375 mm gravity sewer, along with pavement rehabilitation along 59 Avenue and 25 Street.	\$1,110,000	
EX UPG 13	Existing Upgrade	Removal of 450 mm and 525 mm gravity sewer and installation of 600 mm gravity sewer, along with pavement rehabilitation and railway crossing along 59 Avenue.	\$1,920,000	
5 Years (2027 to 2029)				
FUT SER 7, 24, 25	Future Servicing	Installation of new 450 mm, 600 mm, and 675 mm gravity sewers on the northern side of the West Commercial neighbourhood to existing sanitary sewer collection system.	\$2,180,000	
FUT SER 18,23,28,34,4 1	Future Servicing	Installation of new 300 mm and 1,200 mm gravity sewers in eastern Wallacefield.	\$6,690,000	
FUT SER 30	Future Servicing	Installation of new 450 mm gravity sewer connecting existing sanitary sewer collection system in Wallacefield to the proposed new installations in eastern Wallacefield.	\$590,000	





ID <sup>1</sup>	Туре	Description	Cost		
FUT SER 31	Future Servicing	Installation of a new 1,200 mm gravity sewer connecting new eastern Wallacefield installations to existing sanitary sewer collection system in Aurora.	\$3,280,000		
10 Years (2029 to 2034)					
FUT SER 13	Future Servicing	Installation of 375 mm gravity sewer on the northern side of West Commercial connecting to the 5-year proposed installations there from the north.	\$700,000		
FUT SER 27	Future Servicing	Installation of 1,200 mm gravity sewer in The Willows to connect to the 5-year proposed installations in eastern Wallacefield.	\$3,250,000		
FUT SER 29	Future Servicing	Installation of 600 mm gravity sewer in eastern Wallacefield connecting to 5-year proposed installations in eastern Wallacefield from the eastern city limits.	\$810,000		
FUT SER 20, 26, East Trunk 1	Future Servicing	Installation of 1,200 mm East Trunk 1 connecting the existing sanitary sewer collection system from Wigfield Industrial to the WWTP northward.	\$14,390,000		
		20 Years (2034 to 2044)			
FUT UPG 1	Future Upgrade	Once triggered, removal of 900 mm gravity sewer and installation of 1,200 mm gravity sewer, along with pavement rehabilitation along 67 Street to the WWTP.	\$4,110,000		
FUT UPG 2	Future Upgrade	Once triggered, removal of 600 mm gravity sewer and installation of 675 mm gravity sewer through the Husky Oil Refinery land.	\$1,860,000		
FUT UPG 5	Future Upgrade	Once triggered, removal of 200 mm gravity sewer and installation of 300 mm gravity sewer, along with pavement rehabilitation along 29 Street.	\$310,000		
FUT SER 4, 19, 21	Future Servicing	Installation of 375 mm, 750 mm, and 900 mm gravity sewer in Glenn E. Neilson Industrial Park, which connects to an existing sanitary sewer collection system in North Industrial.	\$2,610,000		
FUT SER 17	Future Servicing	Installation of 450 mm gravity sewer from southern part of North Industrial connecting to the proposed East Trunk 1.	\$470,000		
FUT SER 3	Future Servicing	Installation of 450 mm gravity sewer from southern half of Wigfield Industrial connecting to the proposed East Trunk 1.	\$580,000		
FUT SER 35, 36	Future Servicing	Installation of 525 mm and 675 mm gravity sewer on southern side of West Commercial along 75 Avenue connecting northward to existing sanitary sewer collection system in West Commercial.	\$1,060,000		
FUT SER 32	Future Servicing	Installation of 450 mm gravity sewer along 75 Avenue connecting to existing sanitary sewer collection system along 29 Street.	\$580,000		
FUT SER 2, 33	Future Servicing	Installation of 375 mm gravity sewer connecting eastern side of The Willows to the proposed 5-year installations in Wallacefield.	\$490,000		
FUT SER 0, 1, 11, 14, 15, 22	Future Servicing	Installation of 450 mm, 675 mm, 750 mm, 900 mm, and 1,050 mm gravity sewers connecting and running west to east through the Southern Recently Appeared Area to the proposed 10-year installations in The Willows	\$11,220,000		





ID <sup>1</sup>	Туре	Description	Cost		
Ultimate (2044 to Full Build-out)					
FUT SER Lift Station 1	Future Servicing	New lift station to be built on the city boundary beside Range Road 11 in the Northern Recently Annexed Area east to the airport.	\$1,050,000		
FUT SER 6	Future Servicing	Installation of 200 mm forcemain along Range Road 11 southward connecting the proposed Lift Station 1 to the proposed Ultimate gravity sewer running along Township Road 502 and 67 Street.	\$730,000		
FUT SER 8	Future Servicing	Installation of 450 mm gravity sewer in Hill Industrial along 75 Avenue connecting to the existing sanitary sewer collection system along 62 Street.	\$390,000		
FUT SER 5	Future Servicing	Installation of 525 mm gravity sewer connecting the Northern Recently Annexed Area to the existing sanitary sewer collection system in Glenn E. Neilson Industrial Park.	\$410,000		
FUT SER 9, 38, 39, 40, 42	Future Servicing	Installation of 450 mm, 600 mm, and 750 mm gravity sewer along Township Road 502 and 67 Street connecting to the existing sanitary sewer collection system in Glenn E. Neilson Industrial Park.	\$3,480,000		
FUT SER 12	Future Servicing	Installation of 600 mm gravity sewer in the Southern Recently Annexed Area connecting to the proposed 20-year installations in the same area.	\$370,000		
FUT SER 10, 16	Future Servicing	Installation of 450 mm and 600 mm gravity sewer in the Southern Recently Annexed Area connecting to the proposed 20-year installations in the same area.	\$780,000		

<sup>1</sup> The ID numbers for servicing represent pipe ID.



☆	Wastewater Treatment Plant				
•	Proposed Lift Station				
_	Existing Sewer				
C_2	Study Area				
Prop	osed Existing Pipe Upgrade				
Diam	Diameter				
-	250 mm				
	300 mm				
	375 mm				
	450 mm				
	525 mm				
	600 mm				
	900 mm				
Proposed Future Pipe Upgrade					
Diam	eter				
	300 mm				
	675 mm				
Dren	1200 mm				
Diam	otor				
	200 mm Forcemain				
	300 mm Sewer				
-	375 mm Sewer				
-	450 mm Sewer				
-	525 mm Sewer				
-	600 mm Sewer				
-	675 mm Sewer				
-	750 mm Sewer				
-	900 mm Sewer				
-	1050 mm Sewer				
-	1200 mm Sewer				
Futu	e Parcel Tie-In				
	New Service				
////	Existing Service				
	Lift Station Required				
	Septic Tanks				
	Serviced But Vacant				
	Airport Expansion				





- Wastewater Treatment Plant
- Proposed Lift Station
- Existing Sewer
- Study Area

#### Staging

- ---- Existing Upgrades
- Future Upgrades - -
- → 5-Year Growth
- 10-Year Growth
- → 20-Year Growth
- → Ultimate Boundary

## **Development Timing**



3-Years 5-Years 10-Years 20-Years Ultimate Boundary











### 9.0 Conclusions and Recommendations

ISL was retained by the City to provide engineering services to generate an updated SSMP. The SSMP evaluated the current and future performance and capacity of the sanitary sewer collection system in the city, and assessed any additional servicing required to meet the needs of future populations.

Since the last update to the SSMP in 2016, the City has undergone significant changes, including expansion through the 2022 Annexation Lands and various sanitary sewer collection system upgrades. These changes, along with the anticipated increase in sanitary flows from annexation and population growth and the ongoing deterioration of the sanitary sewer collection system, necessitate an updated SSMP. This updated plan will help the City understand and manage the servicing implications of new developments, ensuring effective infrastructure improvements and future expansion while maintaining service levels for residents and businesses.

The purpose of the SSMP is generally summarized below:

- Inventory and analyze the existing infrastructure under existing conditions;
- Convert and update the City's existing hydraulic model;
- Calibrate the City's hydraulic model under dry weather flow (DWF) and wet weather flow (WWF) conditions to accurately represent the flow conditions within the City's existing sanitary sewer collection system;
- Use the calibrated hydraulic model to prepare capacity assessments of the existing sanitary sewer collection system under current and future growth conditions;
- Develop servicing plans for future growth. Locations and timing may be dependent on the following:
  - Availability of sufficient servicing needs;
  - Annexed land locations; and
  - · Community planning;
- Determine what upgrades are required to the existing sanitary sewer collection system based on condition and capacity assessments and recommend future servicing options; and
- Provide a framework for future sanitary sewer collection system capital planning, including cost estimates and possible staging of infrastructure installations.

Conclusions and recommendations from the updated SSMP are provided in Sections 9.1 and 9.2 below for the sanitary sewer collection system.





#### 9.1 Sanitary System Conclusions

Conclusions for the sanitary sewer collection system are as follows:

- Significant inflow and infiltration were identified at Sites 7, 9, 12, and 14. Most other sites were within typical design guidelines of 0.28 L/s/ha or less.
- Flow monitoring and rain gauge data collected in 2022 indicated that no major rainfall events occurred during the monitoring period, as all storms were less than a 1:2 year return period.
- DWF and WWF calibration was undertaken for the 2022 monitoring period. Successful calibration was achieved based on agreement between the observed and modelled data.
- The sanitary sewer collection system was evaluated in terms of various return period events, with the level of service event identified as the 1:25 year design storm event.
- 13 locations were flagged as areas of concern under the level of service event.
- A CCTV inspection program was undertaken in various areas of the city, covering a range of sewer installation years, materials, and pipe diameters. Pipe conditions were documented and flagged for repairs or maintenance depending on their condition rating.
- A risk assessment was undertaken to prioritize the capacity and condition upgrades recommended under existing sanitary sewer collection system conditions.
- The proposed sanitary sewer collection system is comprised of gravity sewers, lift stations, and forcemains to service future growth areas:
  - Two (2) concepts were evaluated to service future areas: Concept 1 and Concept 2. The main difference between the two (2) concepts is the ultimate tie-in point of one single-family quarter section on the west side of the city;
  - In Concept 1 it is tied-into the existing sanitary infrastructure and in Concept 2 it is routed to the proposed South Trunk; and
  - Concept 1 was selected as the recommended option as it allows greater flexibility regarding staging of the quarter section.
- Under Ultimate Boundary Growth conditions, the existing sanitary sewer collection system was generally found to perform adequately.
  - Some surcharging was noted in seven (7) areas across the two (2) concepts; however, in many cases the surcharging was minor;
  - Upgrades for three (3) of the seven (7) noted areas were recommended; and
  - The future network assumes all the recommended existing sanitary upgrades are implemented. These upgrades should be completed prior to any substantial densification or future development.
- Hydraulic assessment of the proposed sanitary sewer collection system is sufficient in managing sewage generated from the future development areas given that all proposed upgrades are implemented.





#### 9.2 Sanitary System Recommendations

Recommendations for the sanitary sewer collection system are as follows:

- Prioritize upgrades to the existing sanitary sewer collection system based on the order documented in Table 6.11:
  - To mitigate surcharging in the existing sanitary sewer collection system, implement the necessary upgrades recommended under the LOS design storm scenario; and
  - Remediate the pipes flagged as being in poor condition from the CCTV inspections.
- Continue condition assessments and flow monitoring and aligning infrastructure upgrades with development and roadworks programs to minimize costs.
- Conduct an inflow-infiltration field investigation program to pinpoint the sources of I-I. Field investigation could consist of smoke testing, micro flow monitoring, dye testing and CCTV inspections:
  - Once the field investigation is completed and areas of sources of I-I have been identified, these issues should be mitigated; and
  - Additional flow monitoring and WWF calibration would be required following the program to update the runoff and I-I model parameters.
- Preferred servicing future concept recommended is Concept 1. Recommended staging of Concept 1 for the 5-, 10-, and 20-Year Growth Horizons are shown in Figures 7.12 to 7.14, respectively:
  - The future sanitary sewer collection system should be designed based on the City's Municipal Development Standards.
- The SSMP should be reviewed and updated after significant periods of growth or every five (5) years to update the hydrodynamic model and analysis with any capital upgrades completed by the City, and the most up-to-date growth plans. This could provide clarity on the planned location of development, the density of the proposed development, and the potential corresponding upgrades. This will ensure capacity is maintained and staging upgrades are advancing as needed.





### **10.0** References

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19, 2023)





**APPENDIX**Dry Weather Flow Diurnals





FIGURE A.1 DIURNAL PLOT SITE 1 - RESIDENTIAL SANITARY SEWER MASTER PLAN















FIGURE A.3 DIURNAL PLOT SITE 2 - NON-RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.4 DIURNAL PLOT SITE 3 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.5 DIURNAL PLOT SITE 4 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.6 DIURNAL PLOT SITE 5 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.7 DIURNAL PLOT SITE 5 - NON-RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.8 DIURNAL PLOT SITE 6 - RESIDENTIAL SANITARY SEWER MASTER PLAN















FIGURE A.10 DIURNAL PLOT SITE 8 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.11 DIURNAL PLOT SITE 9 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.12 DIURNAL PLOT SITE 10 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.13 DIURNAL PLOT SITE 11 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.14 DIURNAL PLOT SITE 12 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.15 DIURNAL PLOT SITE 13 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.16 DIURNAL PLOT SITE 13 - NON-RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.17 DIURNAL PLOT SITE 14 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.18 DIURNAL PLOT SITE 14 - NON-RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.19 DIURNAL PLOT SITE 15 - RESIDENTIAL SANITARY SEWER MASTER PLAN







FIGURE A.20 DIURNAL PLOT SITE 15 - NON-RESIDENTIAL SANITARY SEWER MASTER PLAN











**APPENDIX**Dry Weather Flow Calibration Graphs










FIGURE B.2 DWF CALIBRATION SITE 2 SANITARY SEWER MASTER PLAN







FIGURE B.3 DWF CALIBRATION SITE 3 SANITARY SEWER MASTER PLAN















FIGURE B.5 DWF CALIBRATION SITE 5 SANITARY SEWER MASTER PLAN







FIGURE B.6 DWF CALIBRATION SITE 6 SANITARY SEWER MASTER PLAN























FIGURE B.9 DWF CALIBRATION SITE 9 SANITARY SEWER MASTER PLAN















FIGURE B.11 DWF CALIBRATION SITE 11 SANITARY SEWER MASTER PLAN















FIGURE B.13 DWF CALIBRATION SITE 13 SANITARY SEWER MASTER PLAN















FIGURE B.15 DWF CALIBRATION SITE 15 SANITARY SEWER MASTER PLAN











**APPENDIX** Wet Weather Flow Calibration Graphs



FIGURE C.1 WWF CALIBRATION - PRIMARY EVENT SITE 1 SANITARY SEWER MASTER PLAN







FIGURE C.2 WWF CALIBRATION - PRIMARY EVENT SITE 2 SANITARY SEWER MASTER PLAN







FIGURE C.3 WWF CALIBRATION - PRIMARY EVENT SITE 3 SANITARY SEWER MASTER PLAN







FIGURE C.4 WWF CALIBRATION - PRIMARY EVENT SITE 4 SANITARY SEWER MASTER PLAN







FIGURE C.5 WWF CALIBRATION - PRIMARY EVENT SITE 5 SANITARY SEWER MASTER PLAN







FIGURE C.6 WWF CALIBRATION - PRIMARY EVENT SITE 6 SANITARY SEWER MASTER PLAN







FIGURE C.7 WWF CALIBRATION - PRIMARY EVENT SITE 7 SANITARY SEWER MASTER PLAN







FIGURE C.8 WWF CALIBRATION - PRIMARY EVENT SITE 8 SANITARY SEWER MASTER PLAN







FIGURE C.9 WWF CALIBRATION - PRIMARY EVENT SITE 9 SANITARY SEWER MASTER PLAN







FIGURE C.10 WWF CALIBRATION - PRIMARY EVENT SITE 10 SANITARY SEWER MASTER PLAN







FIGURE C.11 WWF CALIBRATION - PRIMARY EVENT SITE 11 SANITARY SEWER MASTER PLAN







FIGURE C.12 WWF CALIBRATION - PRIMARY EVENT SITE 12 SANITARY SEWER MASTER PLAN







FIGURE C.13 WWF CALIBRATION - PRIMARY EVENT SITE 13 SANITARY SEWER MASTER PLAN







FIGURE C.14 WWF CALIBRATION - PRIMARY EVENT SITE 14 SANITARY SEWER MASTER PLAN







FIGURE C.15 WWF CALIBRATION - PRIMARY EVENT SITE 15 SANITARY SEWER MASTER PLAN







FIGURE C.16 WWF CALIBRATION - VALIDATION EVENT SITE 1 SANITARY SEWER MASTER PLAN







FIGURE C.17 WWF CALIBRATION - VALIDATION EVENT SITE 2 SANITARY SEWER MASTER PLAN







FIGURE C.18 WWF CALIBRATION - VALIDATION EVENT SITE 3 SANITARY SEWER MASTER PLAN







FIGURE C.19 WWF CALIBRATION - VALIDATION EVENT SITE 4 SANITARY SEWER MASTER PLAN







FIGURE C.20 WWF CALIBRATION - VALIDATION EVENT SITE 5 SANITARY SEWER MASTER PLAN






FIGURE C.21 WWF CALIBRATION - VALIDATION EVENT SITE 6 SANITARY SEWER MASTER PLAN







FIGURE C.22 WWF CALIBRATION - VALIDATION EVENT SITE 7 SANITARY SEWER MASTER PLAN







FIGURE C.23 WWF CALIBRATION - VALIDATION EVENT SITE 8 SANITARY SEWER MASTER PLAN







FIGURE C.24 WWF CALIBRATION - VALIDATION EVENT SITE 9 SANITARY SEWER MASTER PLAN







FIGURE C.25 WWF CALIBRATION - VALIDATION EVENT SITE 10 SANITARY SEWER MASTER PLAN







FIGURE C.26 WWF CALIBRATION - VALIDATION EVENT SITE 11 SANITARY SEWER MASTER PLAN







FIGURE C.27 WWF CALIBRATION - VALIDATION EVENT SITE 12 SANITARY SEWER MASTER PLAN







FIGURE C.28 WWF CALIBRATION - VALIDATION EVENT SITE 13 SANITARY SEWER MASTER PLAN







FIGURE C.29 WWF CALIBRATION - VALIDATION EVENT SITE 14 SANITARY SEWER MASTER PLAN







FIGURE C.30 WWF CALIBRATION - VALIDATION EVENT SITE 15 SANITARY SEWER MASTER PLAN









D

**APPENDIX** Longitudinal Profiles



- 25 Year Water Level
- Ground Elevation



FIGURE D.1 EXISTING CONDITIONS 25 STREET TRUNK SEWER LP SANITARY SEWER MASTER PLAN







- 25 Year Water Level
- Ground Elevation



Credits:Source: Esri, Maxar, Earthstar Geographics, and the GIS User Community

FIGURE D.2 EXISTING CONDITIONS 36 STREET TRUNK SEWER LP SANITARY SEWER MASTER PLAN







- 25 Year Water Level
- Ground Elevation



FIGURE D.3 EXISTING CONDITIONS 47 STREET TRUNK SEWER LP SANITARY SEWER MASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



FIGURE D.4 EXISTING CONDITIONS 52 STREET TRUNK SEWER LP SANITARY SEWER MASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



FIGURE D.5 EXISTING CONDITIONS 62 STREET TRUNK SEWER LP SANITARY SEWER MASTER PLAN 277







### Legend

- 25 Year Water Level
- Ground Elevation



FIGURE D.6 EXISTING CONDITIONS EAST TRUNK SEWER LP SANITARY SEWER MASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



FIGURE D.7 EXISTING CONDITIONS NORTH TRUNK SEWER LP SANITARY SEWER MASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



#### FIGURE D.8 EXISTING CONDITIONS SOUTHEAST TRUNK SEWER LP SANITARY SEWER MASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



FIGURE D.9 EXISTING CONDITIONS WEST TRUNK SEWER LP SANITARY SEWER MASTER PLAN







- 25 Year Water Level
- Ground Elevation



## FIGURE D.10 EXISTING CONDITIONS - LP 1 SANITARY SEWERMASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



## FIGURE D.11 EXISTING CONDITIONS - LP 2 SANITARY SEWERMASTER PLAN







25 Year Water Level
Ground Elevation



#### FIGURE D.12 EXISTING CONDITIONS - LP 3 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



# FIGURE D.13 EXISTING CONDITIONS - LP 4 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



## FIGURE D.14 EXISTING CONDITIONS - LP 5 SANITARY SEWERMASTER PLAN







 25 Year Water Level Ground Elevation



## FIGURE D.15 EXISTING CONDITIONS - LP 6 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



### FIGURE D.16 EXISTING CONDITIONS - LP 7 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



## FIGURE D.17 EXISTING CONDITIONS - LP 8 SANITARY SEWERMASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



#### FIGURE D.19 EXISTING CONDITIONS - LP 10 SANITARY SEWERMASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



#### FIGURE D.19 EXISTING CONDITIONS - LP 10 SANITARY SEWERMASTER PLAN







 25 Year Water Level Ground Elevation



## FIGURE D.20 EXISTING CONDITIONS - LP 11 SANITARY SEWERMASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



#### FIGURE D.21 EXISTING CONDITIONS - LP 12 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



# FIGURE D.22 EXISTING CONDITIONS - LP 13 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



# FIGURE D.23 EXISTING CONDITIONS - LP 14 SANITARY SEWERMASTER PLAN







 25 Year Water Level Ground Elevation



### FIGURE D.24 EXISTING CONDITIONS - LP 15 SANITARY SEWERMASTER PLAN







25 Year Water Level
Ground Elevation



#### FIGURE D.25 EXISTING CONDITIONS - LP 16 SANITARY SEWERMASTER PLAN






- 25 Year Water Level
- Ground Elevation



# FIGURE D.26 EXISTING UPGRADES - LP 1 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



### FIGURE D.27 EXISTING UPGRADES - LP 2 SANITARY SEWERMASTER PLAN







## Legend

 25 Year Water Level Ground Elevation



# FIGURE D.28 EXISTING UPGRADES - LP 3 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



# FIGURE D.29 EXISTING UPGRADES - LP 4 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



# FIGURE D.30 EXISTING UPGRADES - LP 5 SANITARY SEWERMASTER PLAN







### Legend

- 25 Year Water Level
- Ground Elevation



# FIGURE D.31 EXISTING UPGRADES - LP 6 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



FIGURE D.32 EXISTING UPGRADES - LP 7 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



FIGURE D.33 EXISTING UPGRADES - LP 8 SANITARY SEWERMASTER PLAN









LP 9



- 25 Year Water Level
- Ground Elevation



### FIGURE D.35 EXISTING UPGRADES - LP 10 SANITARY SEWERMASTER PLAN







 25 Year Water Level Ground Elevation



# FIGURE D.36 EXISTING UPGRADES - LP 11 SANITARY SEWERMASTER PLAN







Legend

- 25 Year Water Level
- Ground Elevation



FIGURE D.37 EXISTING UPGRADES - LP 12 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



# FIGURE D.38 EXISTING UPGRADES - LP 13 SANITARY SEWERMASTER PLAN







- 25 Year Water Level
- Ground Elevation



# FIGURE D.39 EXISTING UPGRADES - LP 14 SANITARY SEWERMASTER PLAN







25 Year Water Level
 Ground Elevation



### FIGURE D.40 EXISTING UPGRADES - LP 15 SANITARY SEWERMASTER PLAN







 25 Year Water Level Ground Elevation



# FIGURE D.41 EXISTING UPGRADES - LP 16 SANITARY SEWERMASTER PLAN







### Legend

Concept 1 HGL

- Concept 2 HGL

### Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



# FIGURE D.42 FUTURE CONCEPTS - LP 1 SANITARY SEWER MASTER PLAN







- Concept 1 HGL
- Concept 2 HGL

# Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



#### FIGURE D.43 FUTURE CONCEPTS - LP 2 SANITARY SEWER MASTER PLAN







- Concept 1 HGL
- Concept 2 HGL

# Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



#### FIGURE D.44 FUTURE CONCEPTS - LP 3 SANITARY SEWER MASTER PLAN







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		1.
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		820
		150255
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00220	SANMH00126	3
	200000000000000000000000000000000000000	

### Legend

- Concept 1 HGL
- Concept 2 HGL

### Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



FIGURE D.45 FUTURE CONCEPTS - LP 4 SANITARY SEWER MASTER PLAN







- Concept 1 HGL
- Concept 2 HGL

# Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



FIGURE D.46 FUTURE CONCEPTS - LP 5 SANITARY SEWER MASTER PLAN







Concept 1 HGL

Concept 2 HGL

# Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



#### FIGURE D.47 FUTURE CONCEPTS - LP 6/7 SANITARY SEWER MASTER PLAN







### Legend

Concept 1 HGL

- Concept 2 HGL

### Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



# FIGURE D.48 FUTURE UPGRADES - LP 1 SANITARY SEWER MASTER PLAN







Legend

Concept 1 HGL

- Concept 2 HGL

### Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



FIGURE D.49 FUTURE UPGRADES - LP 2 SANITARY SEWER MASTER PLAN







Legend

Concept 1 HGL

- Concept 2 HGL

### Both Concept 1 and 2 HGL are for the 1:25 Year 24-Hour Huff storm.



FIGURE D.50 FUTURE UPGRADES - LP 5 SANITARY SEWER MASTER PLAN









Е

**APPENDIX** CCTV Inspection Reports

### Tabular Report of PSR 2882-2881

FH

**Total Length Surveyed** 

End of Survey

for ISL

Setup		1	Sur	vevor	Ben Cooper		С	ertificate #	U-315	5-0602	3838	3	S١	/stem	Owner		
Draina	ae			-,	Surv	ev Custo	omer	ISL					-,	,			
P/O #	.90				Date 2023-	12-27	т	ime 11.14	ç	Street	49th	Ave					
City		Llove	minet	<b>n</b> r		Eurther	י +בססו	ion dotaile			.001	,					
		сюуа	minste	1		ruither	locat	ion details									
Up	28	882				Rin	n to i	nvert		Grad	e to	inv	ert		Rim	n to grade	М
Down	28	881				Rin	n to i	nvert		Grad	le to	inv	ert		Rim	n to grade	М
Use S	Sani	itary				Direction	n Dov	vn	Flo	w con	trol				Ν	/ledia No	
Shape	С	ircular				Height	200	Width	mn	n	Prec	clea	ı <b>n</b> J		Date	Cleaned	
Materia	al	Polyv	inyl C	hloride			Joint	length	М	Total	leng	gth	99.6	5 <b>M</b>	Leng	gth Surveyed 99.	6 <b>M</b>
Lining							Yea	ar laid	Ye	ar reh	abili	tate	d		Weat	her Dry	
Purpos	se							Cat								Pressure	
Additio	ona	l info										Stru	ictur	ral	O & M	Constructio	onal
Locatio	on											Mise	cella	aneous	6		
Projec	t	Lloy	dmins	ter Pha	se 1									Wo	rk Order		
Northi	ng							Easting						El	evation		
Coordi	inat	te Sys	stem					5				G	PS .	Accur	acy		
Count \	/ide	90	CD (	Code				In1	ln2	%	.Int	Fr	Το	ImRef	f Remarks		
				CT CT	Start of Surv	201/				/0							
					Markala	су									2882		
0.0				AMH	iviannole					10					2002		
0.0				MWL	Water Level					10							
99.6				AMH	Manhole										2881		

 Scores
 Structural:
 Pipe Rating 0
 Pipe Ratings Index 0
 Peak 0
 Mean Pipe 0

 O&M:
 Pipe Rating 0
 Pipe Ratings Index 0
 Peak 0
 Mean Pipe 0



99.6

99.6 **M** 

### Pipe Graphic Report of PSR 2882-2881

for ISL

Setup 1 Surveyor	Ben Cooper	Certificate #	U-315-0602383	8 System Ov	wner	
Drainage	Survey Custo	mer ISL				
P/O #	Date 2023-02-27	<b>Time</b> 11:14	Street 49	th Ave		
City Lloydminster	Further	location details	5			
<b>Up</b> 2882	Rir	n to invert	Grade	o invert	Rim to	grade M
<b>Down</b> 2881	Rir	n to invert	Grade f	o invert	Rim to	grade M
Use Sanitary	Directio	n Downstream	Flow contro	bl	Medi	a No
Shape Circular	Height	200 Width	mm Pr	eclean J	Date Cle	aned
Material Polyvinyl Chloride	9	Joint length	M Total le	ngth 99.6 M	Length	Surveyed 99.60 M
Lining		Year laid	Year rehab	ilitated	Weather	Dry
Purpose		C	Cat			
Additional info				Structural	O & M	Constructional
Location				Miscellaneous	Hydraulic	
Project Lloydminster Ph	ase 1			Worl	k Order	
Northing		Easting	J	Elev	vation	
Coordinate System				GPS Accura	су	





### Tabular Report of PSR 85-84

for ISL

Tabular Rep	on of PSR	63-64		IOr	13							
Setup 2	Surveyor	Ben Cooper	Certificate #	U-315-0	6023	383	В	S	ystem	Owner		
Drainage		Survey Custon	ner ISL									
P/O #		Date 2023-02-27	Time 12:47	Str	eet 4	49th	Ave	Э				
City Llo	ydminster	Further Ic	ocation details									
<b>Up</b> 85		Rim	to invert	G	Grad	e to	inv	ert		Rim to	grade	М
Down 84		Rim	to invert	G	Grad	e to	inv	ert		Rim to	grade	М
Use Sanitary	1	Direction	Down	Flow	con	trol				Med	lia No	
Shape Circu	lar	Height 2	00 Width	mm		Pre	clea	an .	J	Date Cl	eaned 2023-02-3	27
Material Vit	rified Clay Pipe	J	oint length	мт	otal	len	gth	100	).2 <b>M</b>	Length	Surveyed 100.	2 M
Lining			Year laid	Year	reha	abili	itate	ed		Weather	· Dry	
Purpose			Cat								Pressure	
Additional in	fo						Stru	Jctu	ral	O & M	Constructior	nal
Location							Mis	cell	aneous			
Project ∟	oydminster Pha	ase 1							Wor	k Order		
Northing			Easting						Ele	vation		
Coordinate S	System		C C				G	PS	Accura	асу		
	CD Code		In1	In2	0/_	Int	Fr	То	ImRof	Romarks		
		Start of Survey			//			10		Kemarka		
0.0	AMH	Manhole							8	35		$\rightarrow$
0.0	MWL	Water Level			0							$\neg$
1.8	FC	Fracture Circumferentia					09	02				
3.1	FM	Fracture Multiple					03	06				
17.9	CL	Crack Longitudinal					09					
22.8	FC	Fracture Circumferentia				J	12	04				
24.0	В	Broken					06					
35.1	FL	Fracture Longitudinal					06					
54.8	FL	Fracture Longitudinal					07					
79.6	FC	Fracture Circumferentia				J	80	10				
82.1	В	Broken				J	05					
82.2	HVV	Hole Void Visible					04					
89.0	RFL	Roots Fine Lateral					11					
89.6	В	Broken					06					
98.0	MWL	S Water Level Sag			20							

100.2 M Total Length Surveyed

AMH Manhole

End of Survey

FH

100.2

100.2

Scores	Structural:	Pipe Rating 34	Pipe Ratings Index 2.8	Peak 5	Mean Pipe 0.3
	O&M:	Pipe Rating 1	Pipe Ratings Index 1	Peak 1	Mean Pipe 0

84



### Pipe Graphic Report of PSR 85-84

for ISL

Sotup		2	Surveyor	Ben Cooper	Cor	tificato #	11-315	06023838	System O	wnor		
Jeiup		2	Surveyor	Den Oooper	Cei	tineate #	0.010	00020000	System O	WIICI		
Draina	ge			Survey C	Customer	ISL						
P/O #				Date 2023-02-2	7 <b>T</b>	<b>ime</b> 12:47	S	treet 49th	Ave			
City		Lloy	dminster	Fur	ther locat	ion details	5					
Up	85	5			Rim to i	nvert		Grade to	invert	Rim to	grade	М
Down	84	4			Rim to i	nvert		Grade to	invert	Rim to	grade	М
Use S	Sani	itary		Dire	ection Dov	vnstream	Flo	w control		Med	ia No	
Shape	Shape Circular				eight 200	Width	mm	Pre	clean J	Date Cle	aned 2023	-02-27
Materia	Shape Circular flaterial Vitrified Clay Pipe				Joint	length	М	Total leng	gth 100.2 M	Length	Surveyed	100.20 <b>M</b>
Lining					Ye	ar laid	Yea	ar rehabili	tated	Weather	Dry	
Purpos	se					C	at					
Additio	ona	l infe	D						Structural	O & M	Constru	ictional
Locati	on								Miscellaneous	Hydraulic		
Projec	t	Llo	ydminster Ph	ase 1					Wor	k Order		
Northing					Easting	I		Ele	vation			
Northing Coordinate System								GPS Accura	icy			





### Pipe Graphic Report of PSR 85-84

for ISL

Setun		2	Surveyor	Ben Cooper	Cer	rtificate #	11-315	06023838	System O	wner		
		2	Our veyor				0 010	00020000	oystem o	WIICI		
Draina	ge			Survey C	ustomer	ISL						
P/O #				Date 2023-02-27	Т	ime 12:47	S	treet 49th	n Ave			
City		Lloy	dminster	Furt	her locat	ion details	5					
Up	85	5			Rim to i	nvert		Grade to	o invert	Rim to	grade	М
Down	84	ļ			Rim to i	nvert		Grade to	o invert	Rim to	grade	М
Use S	Sani	tary		Dire	ction Dov	wnstream	Flo	w control		Med	lia No	
Shape	Shape Circular				<b>ight</b> 200	Width	mm	Pre	clean J	Date Cle	aned 2023	8-02-27
Materia	Shape Circular <b>/aterial</b> Vitrified Clay Pipe			1	Joint	t length	М	Total len	gth 100.2 M	Length	Surveyed	100.20 <b>M</b>
Lining					Ye	ar laid	Yea	ar rehabil	itated	Weather	Dry	
Purpos	se					C	Cat					
Additio	ona	l info	)						Structural	O & M	Constru	uctional
Locati	on								Miscellaneous	Hydraulic		
Projec	t	Llo	ydminster Ph	ase 1				L	Wor	k Order		
Northing					Easting	J		Ele	vation			
Coordinate System								GPS Accura	acy			





CCTV Picture List of	85-84		for	ISL	
Work Order	Survey Date	,	2023-02-	27	Setup 2
Video	Survey Date	4	2023-02-	21	
Path to picture files	C:\FLEX6\Snaps\IS	L\			
Path to video files	C:\FLEX6\Movies\IS	SL\			
Path to media files	C:\FLEX6\Media\ISI	_\			
Control (1) 2 Control (1) 2 Control (1) Control (1) C	Video Index Code Hole Void Visib	Count	82.2 M		
JURED IN ANY ANY ANY	File Name 17.jpg				
V V	/ideo Index	Count	89.6 M		
and the second sec	Code Broken				
	Remarks				
Ren ar little sta	File Name 18.jpg				



### Tabular Report of PSR 357-356

for ISL

Setup	12	Su	rveyor	Ben Cooper	C	ertifica	te # l	J-315	-06023	3838		Syste	em C	Owner			
Draina	ige		•	Survey	/ Customer	ISL											
P/O #	•			Date 2023-02	-28 <b>T</b>	<b>ime</b> 15	:03	St	treet 4	12nd 3	Stre	et					
Citv	Llo	/dminst	er	F	urther locat	ion det	ails										
dU	357				Rim to i	nvert			Grad	e to i	nve	rt		Rim t	o arade		м
Down	356				Rim to i	nvert			Grad	e to i	nve	rt		Rim t	o grade		м
Use	Sanitary			D	irection Up			Flov	v con	trol	-	-		Ме	dia No		
Shape	Circul	ar			Height 375	Width		mm		Prec	ear	<b>n</b> J		Date C	eaned 20	23-02-2	8
Materi	al Co	ncrete P	ipe (non	-reinforced)	Joint	lenath		м	Total	lena	<b>h</b> 6		л	Lenat	n Surveve	<b>d</b> 68.5	м
Lining			.po (		Ye	ar laid		Yea	r reha	abilitz	atec	1		Weathe	r Drv	<b>u</b> 0010	
Purpo	50				100		Cat	100				•		meane	Prossur	۵	
Addisi		-					Cat					tural		0 8 M	Conor	e tructions	
Additio	onal in	0									Aisce	ellaner	2115	ΟαIVI	Cons	liuciiona	al
Locati	on										11300	charice	543				
Projec	t Lle	bydmins	ster Phas	se 1								v	Vork	c Order			
Northi	ng					East	ting						Elev	vation			
Coord	inate S	ystem									GP	S Acc	ura	су			
Count	Video	CD	Code			l	n1	In2	%	JntF	r T	o ImF	Ref	Remarks			
0.0			ST	Start of Survey	/												
0.0			AMH	Manhole									3	56			
0.0			MWL	Water Level					45								
52.1			SRI	Surface Rough	nness Increas	ed				1	2 1	2					
57.9			MWL	Water Level					25				2	5			Tİ.
68.5			AMH	Manhole									3	57			
68.5			FH	End of Survey													

68.5 M Total Length Surveyed

Scores	Structural:	Pipe Rating 1	Pipe Ratings Index 1	Peak 1	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



### Pipe Graphic Report of PSR 357-356

for ISL

Setup	12	Surveyor	Ben Cooper	(	Certificate #	U-315	5-06023838	System O	wner		
Drainage			Surv	vey Custom	er ISL						
P/O #			Date 2023	-02-28	Time 15:03	9	Street 42nd	d Street			
City	Lloyo	dminster		Further lo	cation details	5					
<b>Up</b> 3	57			Rim t	o invert		Grade to	invert	Rim to	o grade	М
Down 3	56			Rim t	o invert		Grade to	invert	Rim to	o grade	м
Use San	itary			Direction	Jpstream	Flo	w control		Мес	dia No	
Shape C	ircula	r		Height 37	75 Width	mr	n Pre	clean J	Date Cl	eaned 2023	3-02-28
Material	<b>ape</b> Circular <b>terial</b> Concrete Pipe (non-reinforc			Jo	int length	М	Total len	gth 68.5 M	Length	Surveyed	68.50 <b>M</b>
Lining					Year laid	Ye	ar rehabil	itated	Weather	Dry	
Purpose					(	Cat					
Additiona	al info	)						Structural	O & M	Constru	uctional
Location								Miscellaneous	Hydraulic		
Project	Llo	ydminster Ph	ase 1					Worl	k Order		
Northing					Easting	9		Ele	vation		
Coordina	Northing Coordinate System							GPS Accura	су		



### Tabular Report of PSR 356-2882

70.4

70.4 **M** 

FH

**Total Length Surveyed** 

End of Survey

for ISL

Setup		13	Surve	vor	Ben Cooper		C	ertificate #	U-315	-0602	3838	3	S	vstem	Owner		
Draina	nae	-		,	Surv	vev Custo	mer	ISI	2 0.0				•.	,			
P/0 #	.90				Date 2023-	.02-28	т	ime 15.37	9	troot	42nd	Str	۵¢t				
170 #									0	lieer	4211u	Our	001				
City		Lloyd	minster			Further	locat	ion details									
Up	35	6				Rin	n to ir	nvert		Grad	e to	inv	ert		Ri	m to grade	М
Down	28	82				Rin	n to ir	nvert		Grad	e to	inv	ert		Ri	m to grade	М
Use S	Sanit	tary				Direction	n Dov	/n	Flo	w con	trol					Media No	
Shape	e Ci	rcular				Height	375	Width	mm	n	Prec	clea	in J	J	Date	e Cleaned 2023-0	2-28
Materi	al	Conc	rete Pipe	(non	-reinforced)		Joint	length	М	Total	leng	gth	70.	4 M	Ler	ngth Surveyed 70	0.4 <b>M</b>
Lining	I						Yea	ar laid	Ye	ar reh	abili	tate	d		Weat	ther Dry	
Purpo	se							Cat								Pressure	
Additi	ona	l info										Stru	ictu	ral	O & M	Construct	ional
Locati	on											Mis	cella	aneous	6		
Projec	t	Lloy	dminster	Phas	se 1									Wo	rk Order		
Northi	ng							Easting						EI	evation		
Coord	inat	e Sys	stem					•				G	PS	Accu	racy		
Count \	Vide		CD Co	do				In1	In2	%	Int	Fr	То	ImRe	f Romark	c	
	viae			-	<u></u>				112	/0	511		10	IIII/C		3	
0.0			S	ľ	Start of Surv	/ey											
0.0			A	MH	Manhole										356		
0.0			N	1WL	Water Level					0							
70.4	0.0 MWL Water Le			Manhole							ĺ			2882			

 Scores
 Structural:
 Pipe Rating
 0
 Pipe Ratings Index
 0
 Peak
 0
 Mean Pipe
 0

 O&M:
 Pipe Rating
 0
 Pipe Ratings Index
 0
 Peak
 0
 Mean Pipe
 0



### Pipe Graphic Report of PSR 356-2882

for ISL

Setup		13	Surveyor	Ben Cooper	(	Certificate #	U-315	-0602383	B System Ov	wner			
Draina	ge			Surv	vey Custom	er ISL							
P/O #				Date 2023	-02-28	Time 15:37	S	Street 42r	nd Street				
City	I	Lloyc	Iminster		Further location details								
Up	356				Rim to invert		Grade to invert			Rim to grade		М	
Down	<b>/n</b> 2882				Rim to invert			Grade t	o invert	Rim to grade		М	
<b>Use</b> Sanitary					Direction Downstream		Flo	w contro	I	Medi	Media No		
Shape Circular					Height 37	5 Width	mm Preclean J			Date Cle	Date Cleaned 2023-02-28		
Materia	al (	Conc	rete Pipe (no	on-reinforced)	Joint length			M Total length 70.4 M		Length	Length Surveyed 70.40 M		
Lining						Year laid			Year rehabilitated		Weather Dry		
Purpose Cat													
Additic	onal	info	)						Structural	O & M	Construc	ctional	
Locatio	on								Miscellaneous	Hydraulic			
Project	Project Lloydminster Phase 1					Work Order							
Northing					Easting			Elevation					
Coordinate System					GPS Accuracy								




### Tabular Report of PSR 561-544

for ISL

Setup	)	34	Su	rveyor	Ben Cooper	С	ertificate #	U-315	-06023	3838	S	ystem	Owner		
Drain	age				Survey Cu	stomer	ISL								
P/O #					Date 2023-03-03	т	ime 8:04	S	treet 4	42nd \$	Street				
City		Lloyc	lminst	er	Furth	ner locat	ion details								
Up	5	61				Rim to i	nvert		Grad	e to i	nvert		Rim te	o grade	М
Down	<b>1</b> 54	44				Rim to i	nvert		Grad	e to i	nvert		Rim te	o grade	М
Use	San	itary			Direc	tion Up		Flo	w con	trol			Me	dia No	
Shap	e C	ircular			Heig	<b>ght</b> 375	Width	mm	1	Precl	ean	J	Date Cl	eaned 2023-03	-03
Mater	rial	Vitrifi	ed Cla	ay Pipe		Joint	length	М	Total	lengt	<b>h</b> 58	.1 <b>M</b>	Length	<b>Surveyed</b> 58.	1 <b>M</b>
Linin	g					Yea	ar laid	Yea	ar reha	abilita	ted		Weathe	<b>r</b> Dry	
Purpo	ose						Cat							Pressure	
Addit	iona	al info								S	tructu	ıral	O & M	Constructio	nal
Locat	tion									N	liscell	aneous	3		
Proje	ct	Lloy	dmin	ster Pha	se 1							Wo	rk Order		-
North	ing						Easting					El	evation		
Coord	dina	te Sy	stem								GPS	Accu	racy		
Count	Vid	ео	CD	Code			In1	In2	%	Jnt F	r To	ImRe	f Remarks		
0.0				ST	Start of Survey										
0.0				AMH	Manhole								544		
0.0				MWL	Water Level				30						
46.6				В	Broken					0	4				
58.1				AMH	Manhole								561		

58.1 M Total Length Surveyed

FH

58.1

End of Survey

Scores	Structural:	Pipe Rating 3	Pipe Ratings Index 3	Peak 3	Mean Pipe 0.1
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



# Pipe Graphic Report of PSR 561-544

for ISL

Setup		34	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-0602383	8 System O	wner		
Draina	ge			Survey Cust	omer	ISL						
P/O #				Date 2023-03-03	٦	<b>ime</b> 8:04	S	street 42r	nd Street			
City		Lloy	dminster	Furthe	r loca	tion detail	5					
Up	56	1		R	m to i	nvert		Grade t	o invert	Rim te	o grade	М
Down	54	4		R	m to i	nvert		Grade t	o invert	Rim te	o grade	М
Use S	Sanit	ary		Directio	on Up	stream	Flo	w contro	I	Ме		
Shape	Shape Circular			Heigh	<b>t</b> 375	Width	mn	n Pr	eclean J	Date Cl	eaned 2023	3-03-03
Materi	Aaterial Vitrified Clay Pipe				Join	t length	М	Total le	ngth 58.1 M	Length	1 Surveyed	58.10 <b>M</b>
Lining					Ye	ar laid	Ye	ar rehabi	litated	Weathe	<b>r</b> Dry	
Purpo	se					(	Cat					
Additi	onal	info	)						Structural	O & M	Constru	uctional
Locati	on								Miscellaneous	Hydraulic		
Projec	Project Lloydminster Phase 1			ase 1					Wor	k Order		
Northi	Northing					Easting	9		Ele	vation		
Coord	Northing Coordinate System								GPS Accura	icy		





#### Tabular Report of PSR 544-655

ISL for

					-	-						
Setup	35	Surveyor	Ben Cooper	Certificate #	U-315-	06023	3838		System	Owner		
Drainag	je		Survey Custom	er ISL								
P/O #			Date 2023-03-03	Time 8:43	St	reet 4	42nd	Stree	t			
City	Lloyd	minster	Further lo	cation details								
Up	544		Rim t	to invert		Grade	e to	inver	t	Rim t	o grade	М
Down	655		Rim t	to invert		Grade	e to	inver	t	Rim t	o grade	М
Use Sa	anitary		Direction	Down	Flow	/ cont	trol			Ме	dia No	
Shape	Circular		Height 3	75 Width	mm		Prec	lean	J	Date C	leaned 2023-03-	03
Materia	I Vitrifi	ed Clay Pipe	Jo	oint length	м .	Total	leng	jth 72	2.7 <b>M</b>	Lengt	n Surveyed 72.7	M N
Lining				Year laid	Yea	r reha	abilit	ated		Weathe	<b>r</b> Dry	
Purpose	е			Cat							Pressure	
Additio	nal info							Struct	ural	O & M	Constructio	nal
Locatio	'n							Misce	llaneous	5		
Project	Lloy	dminster Pha	ase 1						Wo	rk Order		
Northin	g			Easting					Ele	evation		
Coordin	ate Sys	stem		-				GPS	S Accur	асу		
Count Vi	ideo	CD Code		In1	In2	%	.Jnt	Fr To	o ImRef	Remarks		
0.0		ST	Start of Survey							Romanio		
0.0		AMH	Manhole							544		
0.0		MWL	Water Level			20						
25.5		JOM	Joint Offset Medium									
34.6		СМ	Crack Multiple					10 02	2			
63.4		CL	Crack Longitudinal				J	04				
72.7		AMH	Manhole							655		

72.7 **M Total Length Surveyed** 

AMH Manhole

FH

End of Survey

72.7

72.7

<b>Scores</b>	Structural:	Pipe Rating 6	Pipe Ratings Index 2	Peak 3	Mean Pipe 0.1
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



#### Pipe Graphic Report of PSR 544-655

for ISL

Setup	;	35	Surveyor	Ben Cooper	Cer	tificate #	U-315-	06023838	3 System C	)wner		
Draina	ge			Survey	Customer	ISL						
P/O #				Date 2023-03	-03 <b>T</b>	<b>ime</b> 8:43	S	treet 42n	d Street			
City	I	Lloyo	dminster	F	urther locat	ion details	S					
Up	544	4			Rim to i	nvert		Grade to	o invert	Rim	to grade	М
Down	65	5			Rim to i	nvert		Grade to	o invert	Rim	i to grade	м
Use S	l <b>se</b> Sanitary hape Circular				irection Dov	vnstream	Flo	w contro	I	Ν	ledia No	
Shape	Shape Circular				Height 375	Width	mm	Pre	eclean J	Date	Cleaned 20	23-03-03
Materia	Shape Circular Material Vitrified Clay Pipe				Joint	length	М	Total ler	ngth 72.7 M	Leng	jth Surveye	<b>d</b> 72.70 <b>M</b>
Lining					Ye	ar laid	Yea	ar rehabi	litated	Weath	<b>1er</b> Dry	
Purpos	se					C	Cat					
Additio	onal	info	)						Structural	O & M	Cons	tructional
Locati	on								Miscellaneous	Hydraulic	;	
Projec	Project Lloydminster Phase 1			ase 1					Wo	rk Order		
Northi	Northing					Easting	9		Ele	evation		
Coord	Northing Coordinate System								GPS Accur	асу		





#### Tabular Report of PSR 655-543

for ISL

Setup	)	36	Sur	veyor	Ben Cooper	Ce	ertificate #	ŧ U-315	5-06023	3838	S	ystem	Owner		
Drain	age				Survey Cu	istomer	SL								
P/O #					Date 2023-03-03	Ti	<b>me</b> 9:02	5	Street 4	42nd \$	Street				
City		Lloye	dminste	er	Furth	ner locati	on details	5							
Up	6	55				Rim to in	vert		Grad	e to iı	nvert		Rim	to grade	М
Down	5	43				Rim to in	vert		Grad	e to iı	nvert		Rim	to grade	М
Use	San	itary			Direc	tion Dow	'n	Flo	w con	trol			Me	edia No	
Shape	e C	Circula	r		Heig	ght 375	Width	mn	n	Precl	ean 、	J	Date C	cleaned 2023-03	3-03
Mater	ial	Vitrif	ied Cla	y Pipe		Joint	length	М	Total	lengt	<b>h</b> 26.	.8 <b>M</b>	Lengt	th Surveyed 26	.8 <b>M</b>
Lining	3					Yea	ar laid	Ye	ar reha	abilita	ted		Weath	er Dry	
Purpo	se						Ca	t						Pressure	
Addit	iona	al info	)							S	tructu	Iral	O & M	Constructi	onal
Locat	ion									N	liscell	aneous	5		
Proje	ct	Llo	ydminst	ter Pha	se 1					L		Wo	rk Order		
North	ing						Easting	1				EI	evation		
Coord	dina	te Sv	stem					•			GPS	Accu	racy		
Count	V: 4	,		2			l n 4	10	0/	In 4 🗖	- T-	Im D a	f Domorko		
Count	via	eo		Joue				102	70	JILL	10	шке	Remarks		
0.0				ST	Start of Survey										
0.0				AMH	Manhole								655		
0.0				MWL	Water Level				20						
1.7				В	Broken					0	2				
15.8				FM	Fracture Multiple					0	7 03				
21.9				MWL	Water Level				50				50		

26.8 M Total Length Surveyed

FH

AMH Manhole

End of Survey

26.8

26.8

Scores	Structural:	Pipe Rating 7	Pipe Ratings Index 3.5	Peak 4	Mean Pipe 0.3
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0

543



### Pipe Graphic Report of PSR 655-543

for ISL

Setup		36	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-0602383	8 System (	Owner			
Draina	ge			Survey Cu	stomer	ISL							
P/O #				Date 2023-03-03	1	<b>Fime</b> 9:02	S	Street 42r	nd Street				
City		Lloy	dminster	Furth	er loca	tion details	S						
Up	65	5			Rim to i	nvert		Grade t	o invert	F	Rim to gra	de	М
Down	54	3		I	Rim to i	nvert		Grade t	o invert	F	Rim to gra	de	М
Use S	Ise Sanitary				tion Do	wnstream	Flo	w contro	I		Media N	lo	
Shape	Shape Circular				<b>ht</b> 375	Width	mn	n Pr	eclean J	Da	ate Cleane	<b>d</b> 2023-03	3-03
Materia	Snape Circular Material Vitrified Clay Pipe				Join	t length	М	Total le	ngth 26.8 M	L	ength Sur	veyed 26	.80 <b>M</b>
Lining					Ye	ar laid	Ye	ar rehabi	litated	We	eather Dry		
Purpos	se					(	Cat						
Additio	onal	info	)						Structural	O & N	N	Constructi	onal
Locatio	on								Miscellaneous	s Hydra	aulic		
Project	Project Lloydminster Phase 1			ase 1					Wo	rk Order			
Northing						Easting	9		EI	evation			
Coordi	Northing Coordinate System								GPS Accu	racy			





### Tabular Report of PSR 543-656

ISL for

Setup	3	37	Surv	eyor	Ben Cooper	Ce	rtificate #	ŧ U-315	5-06023	3838	3	Syster	m O	wner			
Draina	ige				Survey Cus	stomer I	SL										
P/O #	-				Date 2023-03-03	Ti	<b>me</b> 9:50	5	Street 4	42nd	l Stre	et					
City	L	loydm	ninster		Furth	er locati	on details	5									
Up	543	3			F	Rim to in	vert		Grad	e to	inve	rt		Rim te	o grade		м
Down	656	6			F	Rim to in	vert		Grad	e to	inve	rt		Rim te	o grade		м
Use S	Sanita	ary			Direct	ion Dow	n	Flo	w con	trol				Me	dia No		
Shape	Circ	cular			Heig	<b>ht</b> 375	Width	mn	n	Pred	clear	n J		Date CI	eaned 20	)23-03-0;	3
Materi	al ∖	/itrified	d Clay	Pipe	•	Joint	length	м	Total	leng	gth 4	1.4 <b>M</b>		Length	n Surveye	<b>d</b> 41.4	м
Lining				·		Yea	r laid	Ye	ar reha	abili <sup>.</sup>	, tateo	ł		Weathe	<b>r</b> Dry		
Purpo	se						Ca	t							Pressu	re	
Additi	onal	info									Struc	tural		O & M	Cons	structiona	al
Locati	on										Misc	ellaneo	us				
Projec	t	Lloyd	minste	er Phas	se 1							w	ork	Order			
Northi	ng						Easting	1				E	Eleva	ation			
Coord	inate	Syst	em				•	•			GF	S Acci	urac	v			
Count	lidad	, , ,		odo			Int	ln 0	0/	Int	C, 1	o Im D	<b>~</b> f	Pomorko			
	vided	<u> </u>		oue	Chamb of Criminary				70	Jnt				Remarks			
0.0					Start of Survey								54	3			-
0.0									0				54	5			
0.0					Water Level				25				25				
18.5				MVVL	Water Level				20				25				$\dashv$
24.9				MWL	Water Level				35				35	0			
41.4				AMH	Manhole								65	Ь			
41.4				FH	End of Survey												

Total Length Surveyed 41.4 **M** 

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



# Pipe Graphic Report of PSR 543-656

for ISL

Setup		37	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-06023838	3 System O	wner		
Draina	ge			Survey Cus	tomer	ISL						
P/O #				Date 2023-03-03	٦	<b>Fime</b> 9:50	S	street 42n	d Street			
City		Lloy	dminster	Furthe	r loca	tion detail	S					
Up	54	3		R	im to i	nvert		Grade to	o invert	Rim to	grade	М
Down	65	6		R	im to i	nvert		Grade to	o invert	Rim to	grade	М
Use S	Ise Sanitary				on Do	wnstream	Flo	w contro	I	Мес	lia No	
Shape	Shape Circular				nt 375	Width	mn	n Pre	eclean J	Date Clo	eaned 2023	3-03-03
Materi	Shape Circular Material Vitrified Clay Pipe				Join	t length	М	Total ler	ngth 41.4 M	Length	Surveyed	41.40 <b>M</b>
Lining					Ye	ar laid	Ye	ar rehabi	litated	Weather	Dry	
Purpo	se					(	Cat					
Additi	onal	info	)						Structural	O & M	Constru	uctional
Locati	on								Miscellaneous	Hydraulic		
Projec	Project Lloydminster Phase 1			ase 1					Wor	k Order		
Northi	Northing					Easting	9		Ele	vation		
Coord	Northing Coordinate System								GPS Accura	асу		





#### Tabular Report of PSR 656-542

for ISL

Satur				Pon Coonor	Cortifie	oto #	11 215	0602	2020		Suctor	Owner		
Setup	38	Surv	eyor	Den Cooper	Certific	ate #	0-315	-0602	5038		system	Owner		
Draina	ige			Survey Custo	omer ISL									
P/O #				Date 2023-03-03	Time 1	0:02	S	treet 4	42nd	Stree	et			
City	Lloy	dminster		Further	location de	etails								
Up	656			Rir	n to invert			Grad	e to	invei	t	Rim	to grade	М
Down	542			Rir	n to invert			Grad	e to i	invei	ť	Rim	to grade	М
Use S	Sanitary			Directio	<b>n</b> Down		Flov	w con	trol			M	Media No	
Shape	Circula	ar		Height	375 Wid	th	mm		Prec	lean	J	Date C	Cleaned 2023-03	-03
Materi	<b>al</b> Vitr	fied Clay	Pipe		Joint leng	th	М	Total	leng	th 5	3.8 <b>M</b>	Leng	th Surveyed 58.	8 <b>M</b>
Lining					Year laid	1	Yea	ar reha	abilit	ated		Weath	<b>er</b> Dry	
Purpo	se					Cat							Pressure	
Additi	onal inf	0								Struct	tural	O & M	Constructio	onal
Locati	on									Visce	llaneous	5		
Projec	t Llo	ydminste	er Phas	se 1							Wo	rk Order		
Northi	ng				Ea	sting					EI	evation		
Coord	inate S	ystem								GP	S Accu	racy		
Count	Video	CD C	ode			In1	In2	%	Jnt	Fr To	o ImRe	f Remarks		
0.0			ST	Start of Survey		1								
0.0			AMH	Manhole								656		
0.0			MWL	Water Level				20						
12.0			JSM	Joint Separated Media	um									
16.5			JOM	Joint Offset Medium										
58.8			AMH	Manhole		1						542		
58.8			FH	End of Survey		1								

58.8 M Total Length Surveyed

Scores	Structural:	Pipe Rating 2	Pipe Ratings Index 1	Peak 1	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



# Pipe Graphic Report of PSR 656-542

for ISL

Setup		38	Surveyor	Ben Cooper	Cer	tificate #	U-315	06023838	3 System O	wner		
Draina	ge			Survey	Customer	ISL						
P/O #				Date 2023-03-0	03 <b>T</b>	<b>ime</b> 10:02	S	treet 42n	d Street			
City		Lloyo	dminster	Fu	rther locat	ion details	5					
Up	65	6			Rim to i	nvert		Grade to	o invert	Rim to	grade	М
Down	54	2			Rim to i	nvert		Grade to	o invert	Rim to	grade	М
Use S	Sanit	ary		Dir	ection Dov	vnstream	Flo	w contro	I	Med	ia No	
Shape	Ci	rcula	r	н	eight 375	Width	mm	Pre	eclean J	Date Cle	aned 2023-0	03-03
Materia	al	Vitrif	ied Clay Pipe		Joint	length	Μ	Total ler	ngth 58.8 M	Length	Surveyed 5	8.80 M
Lining					Yea	ar laid	Yea	ar rehabi	litated	Weather	Dry	
Purpos	se					C	at					
Additio	onal	info	)						Structural	O & M	Construc	tional
Locati	on								Miscellaneous	Hydraulic		
Projec	t	Llo	ydminster Ph	ase 1					Wor	k Order		
Northi	ng					Easting	I		Ele	vation		
Coord	Coordinate System								GPS Accura	су		





#### Tabular Report of PSR 542-461

for ISL

Setup	39	)	Surve	yor	Ben Cooper	С	ertificate #	U-315	-06023	3838	5	System	Owner		
Drain	age				Survey Cust	omer	ISL								
P/O #					Date 2023-03-03	т	<b>ime</b> 10:44	S	treet 4	12nd	Stree	t			
City	LI	oydn	ninster		Furthe	r locat	ion details								
Up	542				R	im to i	nvert		Grade	e to i	nver	t	Rim te	o grade	М
Down	461				R	im to i	nvert		Grade	e to i	nver	t	Rim te	o grade	М
Use	Sanita	у			Directio	on Dov	vn	Flov	v con	trol			Me	dia No	
Shape	e Circ	ular			Heigh	<b>t</b> 375	Width	mm		Prec	lean	J	Date Cl	eaned 2023-03	-03
Mater	ial Vi	trifie	d Clay F	⊃ipe		Joint	length	М	Total	leng	<b>th</b> 47	.5 <b>M</b>	Length	Surveyed 47.	5 M
Lining	9					Ye	ar laid	Yea	ar reha	abilit	ated		Weathe	<b>r</b> Dry	
Purpo	ose						Cat							Pressure	
Addit	ional i	nfo								5	Struct	ural	O & M	Constructio	nal
Locat	ion									ſ	/liscel	laneous	3		
Proje	ct l	loyd	Iminster	Pha	se 1							Wo	rk Order		
North	ing						Easting					EI	evation		
Coord	dinate	Syst	tem								GPS	Accu	racy		
Count	Video		CD Co	de			In1	In2	%	Jnt	Fr To	ImRe	f Remarks		
0.0	Tidoo			T	Start of Survey										
0.0			A	MH	Manhole								542		
0.0			N	/WL	Water Level				40						
21.4			В	3	Broken						10				
47.5			A	MH	Manhole								461		

47.5 M Total Length Surveyed

FH

End of Survey

47.5

Scores	Structural:	Pipe Rating 3	Pipe Ratings Index 3	Peak 3	Mean Pipe 0.1
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



# Pipe Graphic Report of PSR 542-461

for ISL

Setup	;	39	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-0602383	8 System (	Owner			
Drainag	ge			Surv	vey Customer	ISL							
P/O #				Date 2023-	03-03 <b>1</b>	<b>Time</b> 10:44	s	Street 42	nd Street				
City	I	loyo	dminster		Further locat	tion details	5						
Up	542	2			Rim to i	nvert		Grade t	o invert		Rim to	grade	М
Down	46 <sup>-</sup>	1			Rim to i	nvert		Grade t	o invert		Rim to	grade	М
Use S	anita	ary			Direction Do	wnstream	Flo	w contro	)		Media	a No	
Shape	Cir	cula	r		Height 375	Width	mn	n Pr	eclean J	[	Date Clea	aned 2023-	03-03
Materia	ıl '	Vitrifi	ied Clay Pipe	•	Join	t length	М	Total le	ngth 47.5 M		Length S	Surveyed 4	7.50 <b>M</b>
Lining					Ye	ar laid	Ye	ar rehab	ilitated	v	leather [	Dry	
Purpos	e					C	at						
Additio	nal	info	)						Structural	0&	М	Construc	tional
Locatio	on								Miscellaneous	B Hyd	raulic		
Project		Lloy	ydminster Ph	ase 1					Wo	rk Orde	er		
Northin	ng					Easting	I		El	evation	1		
Coordinate System							GPS Accu	acy					





#### Tabular Report of PSR 389-388

for ISL

-	•												
Setup	30	Surveyo	or Ben Cooper	Certifi	cate #	U-315-	06023	3838	S	System	Owner		
Draina	age		Survey Cus	tomer ISL									
P/O #			Date 2023-03-02	Time	16:33	St	reet	36th S	treet				
City	Lloy	dminster	Furthe	r location d	etails								
Up	389		R	im to invert			Grad	e to iı	nver	t	Rim	o grade	М
Down	388		R	im to invert			Grad	e to iı	nver	t	Rim	o grade	М
Use	Sanitary		Directi	on Up		Flov	v con	trol			Ме	dia No	
Shape	e Circula	r	Heigh	nt 375 Wid	th	mm		Precl	ean	J	Date C	leaned 2023-03-	02
Mater	ial Con	crete Pipe (r	non-reinforced)	Joint leng	th	М	Total	lengt	h 99	.3 <b>M</b>	Lengt	h Surveyed 99.3	3 <b>M</b>
Lining	J			Year lai	d	Yea	r reha	abilita	ted		Weathe	er Damp	
Purpo	se				Cat							Pressure	
Additi	onal info	)						S	truct	ural	O & M	Constructio	nal
Locat	ion							N	lisce	llaneous	;		
Projec	t Llo	ydminster P	hase 1					·		Wo	rk Order		
North	ing			Ea	asting					El	evation		
Coord	linate Sy	stem			_				GPS	S Accur	acy		
Count	Video	CD Code	)		In1	In2	%	<b>JntF</b>	r To	ImRe	Remarks		
0.0		ST	Start of Survey										
0.0		AM	H Manhole								388		
0.0		MW	L Water Level				45						
5.6		MG	O General Observation								388a		
14.8		SRI	Surface Roughness	Increased				1	2				
94.8		FL	Fracture Longitudina	1				0	2				
99.3		AM	H Manhole								389		

99.3 M Total Length Surveyed

FH

End of Survey

99.3

<b>Scores</b>	Structural:	Pipe Rating 4	Pipe Ratings Index 2	Peak 3	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



### Pipe Graphic Report of PSR 389-388

for ISL

Setup	3	0	Surveyor	Ben Cooper		Certificate #	U-31	5-0602383	B System C	)wner		
Draina	ge			Surv	vey Custom	er ISL						
P/O #				Date 2023	-03-02	Time 16:33	3 :	Street 36t	h Street			
City	L	loyd	minster		Further lo	cation detail	S					
Up	389				Rim	o invert		Grade t	o invert	Rim t	o grade	М
Down	388				Rim	o invert		Grade t	o invert	Rim t	o grade	м
Use S	Sanita	ry			Direction	Upstream	Flo	ow contro	I	Ме	dia No	
Shape	Shape Circular				Height 3	75 Width	mi	n Pr	e <b>clean</b> J	Date C	leaned 202	3-03-02
Materia	Material Concrete Pipe (non-reinforced				Je	oint length	М	Total le	ngth 99.3 M	Lengt	h Surveyed	99.30 <b>M</b>
Lining						Year laid	Ye	ear rehabi	litated	Weathe	r Damp	
Purpos	se					(	Cat					
Additio	onali	info							Structural	O & M	Constr	uctional
Locatio	on								Miscellaneous	Hydraulic		
Project	t	Lloy	dminster Ph	ase 1					Wo	rk Order		
Northi	ng					Eastin	g		Ele	evation		
Coordi	Northing Coordinate System								GPS Accur	асу		





# Tabular Report of PSR 390-389

94.9

94.9 **M** 

MSA

**Total Length Surveyed** 

for ISL

Setup	31	Surveyor	Ben Cooper	C	ertificate #	U-31	5-0602	3838	S	ystem	Owner		
Drainage	•		Survey Cus	stomer	ISL								
P/O #			Date 2023-03-02	т	<b>ime</b> 16:59	5	Street	36th Sti	reet				
City	Lloyo	Iminster	Furth	er locat	ion details								
Up :	390		F	Rim to in	nvert		Grad	e to in	vert		Rim t	o grade	М
Down 🗧	389		F	Rim to in	nvert		Grad	e to in	vert		Rim t	o grade	М
Use Sa	nitary		Direct	<b>ion</b> Dow	/n	Flo	w con	trol			Ме	dia No	
Shape (	Circula		Heig	<b>ht</b> 375	Width	mn	n	Precle	an 、	J	Date C	eaned 2023-03-	02
Material	Cond	rete Pipe (nor	n-reinforced)	Joint	length	М	Total	length	1	М	Lengtl	n Surveyed 94.9	M
Lining				Yea	ar laid	Ye	ar reha	abilitat	ed		Weathe	<b>r</b> Damp	
Purpose					Cat							Pressure	
Addition	al info	)						Sti	uctu	ral	O & M	Construction	nal
Location	1							Mi	scell	aneous	6		
Project	Lloy	dminster Pha	se 1							Wo	rk Order		
Northing	I				Easting					EI	evation		
Coordina	ate Sy	stem						C	<b>SPS</b>	Accu	racy		
Count Vic	leo	CD Code			In1	In2	%	<b>JntFr</b>	То	ImRe	f Remarks		
0.0		ST	Start of Survey										
0.0		AMH	Manhole								390		
1.0		MWL	Water Level				25						
4.9		SAP	Surface Aggregate	Projectin	g			12	1				

Abandoned Survey

Scores	Structural:	Pipe Rating 3	Pipe Ratings Index 3	Peak 3	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0

Manhole insight



# Pipe Graphic Report of PSR 390-389

for ISL

Setup	3	31	Surveyor	Ben Cooper	(	Certificate #	U-31	5-0602383	88 Sys	tem Own	er			
Draina	ge		•	Surv	vey Custom	er ISL			-					
P/O #				Date 2023	-03-02	Time 16:59		Street 36	th Street					
City	L	loyd	minster		Further loc	cation details	5							
Up	390	)			Rim t	o invert		Grade	to invert		Rim to	o grade	М	
Down	389	)			Rim t	o invert		Grade	to invert		Rim to	o grade	М	
Use S	Sanita	iry			Direction [	Downstream	Fle	ow contro	ol		Mee	dia No		
Shape	Shape Circular				Height 37	5 Width	m	m Pi	r <b>eclean</b> J		Date Cl	eaned 2023	3-03-02	
Materia	<b>Aaterial</b> Concrete Pipe (non-reinforced			on-reinforced)	Jo	int length	М	Total le	ngth	М	Length	Surveyed	94.90	м
Lining						Year laid	Ye	ear rehab	ilitated		Weather	Damp		
Purpos	se					(	Cat							
Additic	onal	info							Structur	al	O & M	Constr	uctional	
Locatio	on								Miscella	neous	Hydraulic			
Project	t	Lloy	dminster Ph	ase 1					·	Work C	rder			
Northin	Northing				Easting	9			Eleva	ion				
Coordi	Northing Coordinate System								GPS /	Accuracy				





### Tabular Report of PSR 429-390

for ISL

Setup	32	Surve	vor E	Ben Cooper		C	ertifica	te #	U-315	5-06023	383	8	S	vstem	Owner		
Draina	ae		,	Surv	ev Custo	omer	ISL					-	-	,			
P/O #	.90			Date 2023-0	13-02	т	<b>imo</b> 19	·10	9	troot '	36th	Str	eet				
		dminator			Eurther	laaat	ine dot				5011		001				
		aminster			Further	locat	ion det	ans									
Up	429				Rin	n to ii	nvert			Grad	e to	o inv	/ert		Rin	n to grade	М
Down	390				Rir	n to iı	nvert			Grad	e to	o inv	/ert		Rin	n to grade	М
Use S	Sanitary				Directio	<b>n</b> Up			Flo	w con	trol				N	Media No	
Shape	Circul	ar			Height	375	Width	ľ	mn	า	Pre	clea	an 、	J	Date	Cleaned 2023-03-	-02
Materia	al Cor	crete Pipe	(non-	reinforced)		Joint	length	r	М	Total	len	gth	21.	2 M	Len	gth Surveyed 21.3	2 <b>M</b>
Linina						Yea	ar laid		Ye	ar reha	abil	itate	ed		Weat	her Damp	
Purpos	Purpose							Cat								Pressure	
Additio	onal inf							Str	uctu	ral	0 & M	Constructio	nal				
Loooti		0									Miscellaneous						
Locati	on										L	TTTC					
Projec	t Llo	bydminster	Phase	e 1										Wo	rk Order		
Northi	ng						East	ting						El	evation		
Coord	inate S	ystem										G	PS	Accu	racy		
Count \	/ideo	CD Co	de				1	n1	In2	%	Jn	tFr	То	ImRe	f Remarks	5	
0.0		S	Т	Start of Surv	ev						1						
0.0		A	MH	Manhole	-,						1				390		
0.0		M	WL	Water Level						30							
11.0		M	IGO	General Obs	ervation										Rapid grade	change	
14.6		M	WL	Water Level						15	1				15		
21.2		A	МН	Manhole											429		
21.2		F	Н	End of Surve	ey				1								

21.2 M Total Length Surveyed

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



# Pipe Graphic Report of PSR 429-390

for ISL

Setup	;	32	Surveyor	Ben Cooper		Certificate #	U-31	5-0602383	B System C	Owner		
Draina	ge			Surv	vey Custom	er ISL						
P/O #				Date 2023	-03-02	Time 19:10		Street 36t	h Street			
City	I	Lloyc	Iminster		Further lo	cation detail	s					
Up	429	9			Rim	o invert		Grade t	o invert	Rin	n to grade	М
Down	39	0			Rim	o invert		Grade t	o invert	Rin	n to grade	м
Use S	Sanit	ary			Direction	Jpstream	Flo	ow contro	I	Ν	/ledia No	
Shape	Cir	cular			Height 3	75 Width	mr	n Pro	e <b>clean</b> J	Date	Cleaned 20	23-03-02
Materia	al	Conc	rete Pipe (no	on-reinforced)	Jo	oint length	М	Total ler	ngth 21.2 M	Leng	gth Surveye	<b>d</b> 21.20 <b>M</b>
Lining						Year laid	Ye	ear rehabi	litated	Weat	<b>her</b> Damp	
Purpos	se					(	Cat					
Additio	onal	info	)						Structural	O & M	Cons	tructional
Locati	on								Miscellaneous	Hydrauli	с	
Projec	t	Lloy	dminster Ph	ase 1					Wo	rk Order		
Northi	ng					Easting	g		Ele	evation		
Coordi	Coordinate System								GPS Accur	асу		





#### Tabular Report of PSR 434-429

for ISL

	•												
Setup 3	33 <b>S</b>	urveyor	Ben Cooper	С	ertificate #	U-315-	06023	3838	Sy	stem	Owner		
Drainage			Surve	y Customer	ISL								
P/O #			Date 2023-03	3-02 <b>T</b>	<b>'ime</b> 19:15	St	reet 3	36th St	reet				
City I	Lloydmins	ster	Ĩ	urther locat	ion details								
<b>Up</b> 434	4			Rim to i	nvert		Grade	e to in	vert		Rim t	o grade	М
Down 429	9			Rim to i	nvert		Grade	e to in	vert		Rim t	o grade	М
Use Sanita	ary		0	Direction Up		Flov	v con	rol			Ме	dia No	
Shape Cire	cular			Height 375	Width	mm		Precle	<b>an</b> J		Date C	eaned 2023-03	-02
Material (	Concrete	Pipe (nor	n-reinforced)	Joint	length	м	Total	length	1	М	Lengtl	n Surveyed 8.0	М
Lining				Ye	ar laid	Yea	r reha	abilitat	ed		Weathe	<b>r</b> Damp	
Purpose					Cat							Pressure	
Additional	info							St	ructur	al	O & M	Constructio	nal
Location								Mi	scella	neous			
Project	Lloydmir	nster Pha	se 1							Woi	k Order		
Northing	-				Easting					Ele	evation		
Coordinate	e Systen	า			•			C	GPS /	Accur	acy		
Count Vido		Cada			l n 4	1-0	0/	Int Ca	Та	lun D of	Domoriko		
Count video		Code				Inz	70	Jnter	10	Imker	Remarks		
0.0		ST	Start of Surve	у									
0.0		AMH	Manhole								429		
0.0		MWL	Water Level				20						
0.8		MMC	Material chan	qe							CMP		
6.8		MGO	General Obse	rvation							Pipe invert sus	pected broken	

8.0 M Total Length Surveyed

MSA Abandoned Survey

8.0

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0

Pipe rough, risky to proceed



### Pipe Graphic Report of PSR 434-429

for ISL

Setup	33	Survevor	Ben Cooper	C	ertificate #	U-31	5-0602383	8 Svs	tem Own	ər		
Drainage		•••••	Sur	- vev Custome	er ISL							
P/O #			Date 2023	-03-02	Time 19:15	:	Street 36t	h Street				
City	Lloy	dminster		Further loc	ation details	5						
Up 4	34			Rim to	o invert		Grade t	o invert		Rim to	grade	М
Down 4	29			Rim to	o invert		Grade t	o invert		Rim to	grade	М
Use Sar	nitary			Direction L	pstream	Flo	ow contro	l		Mec	lia No	
Shape C	Circula	r		Height 37	5 Width	mr	n Pr	<b>eclean</b> J		Date Cl	eaned 2023	-03-02
Material	Con	crete Pipe (no	on-reinforced)	Jo	nt length	М	Total le	ngth	М	Length	Surveyed	08.00 <b>M</b>
Lining				٢	ear laid	Ye	ear rehabi	litated		Weather	Damp	
Purpose					(	Cat						
Addition	al info	)						Structur	al (	0 & M	Constru	ictional
Location								Miscella	neous H	lydraulic		
Project	Llo	ydminster Ph	ase 1						Work O	rder		
Northing					Easting	9			Elevat	ion		
Coordina	ate Sy	stem						GPS /	Accuracy			





#### Tabular Report of PSR 528-527

for ISL

Tabala			. •	020 02.													
Setup	3	Sur	veyor	Ben Cooper (	Certific	ate #	U-315-	06023	3838		Sy	stem	Owner				
Draina	ge			Survey Customer	· ISL												
P/O #				Date 2023-02-27	Time 1	3:39	St	reet 4	19th /	Ave							
City	Lloy	dminste	ər	Further loca	tion de	etails											
Up	528			Rim to	invert			Grade	e to	inve	ert		Ri	im to g	rade	1	M
Down	UNK			Rim to	invert			Grade	e to	inve	ert		Ri	im to g	rade	1	M
Use S	Sanitary			Direction Do	wn		Flow	/ cont	trol					Media	No		
Shape	Circula	ır		Height 200	Widt	h	mm		Prec	lea	n J		Dat	e Clear	ned 2023-	-02-27	
Materia	<b>al</b> Vitrif	fied Cla	ıy Pipe	Joir	nt lengt	h	м	Total	leng	jth ·	41.3	М	Lei	ngth Su	urveyed 4	41.3	М
Lining				Ye	ear laic	1	Yea	r reha	abilit	ate	d		Wea	ther D	ry		
Purpos	se					Cat								Р	ressure		
Additic	onal info	o							:	Stru	ctura	al	O & M		Constru	ctional	
Locatio	on									Misc	cella	neous					
Project	t Llo	ydmins	ter Pha	se 1								Woi	k Order				
Northin	ng				Ea	sting						Ele	evation				
Coordi	nate Sy	stem								GI	PS A	Accur	асу				
	/ideo	CD	Code			In1	In2	%	Int	Fr '	ΤοΙ	mRof	Remark	(5			
	luco		ST	Start of Survey		1		/0		<u> </u>			Remain				
0.0			AMH	Manhole			_						528				-
0.0			MWL	Water Level				50									
2.3			MWL	Water Level				20				:	20				1
7.0			DAE	Deposits Attached Encrust	ation			15		04							1
19.8			DAE	Deposits Attached Encrust	ation			15		04							1
27.5			DAE	Deposits Attached Encrust	ation			20		04	80						Ì
32.1			DAE	Deposits Attached Encrust	ation			20		04	08						]

41.3 M Total Length Surveyed

DAE

FH

AMH Manhole

End of Survey

Deposits Attached Encrustation

36.8

41.3

41.3

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 15	Pipe Ratings Index 3	Peak 3	Mean Pipe 0.4

15

02

?



#### Pipe Graphic Report of PSR 528-527

for ISL

Setup	3	Surveyor	Ben Cooper	Cer	tificate #	U-315	-06023838	System O	wner		
Draina	ge		Survey Cu	stomer	ISL						
P/O #			Date 2023-02-27	т	' <b>ime</b> 13:39	S	treet 49th	Ave			
City	L	loydminster	Furth	er locat	ion details	6					
Up	528			Rim to i	nvert		Grade to	invert	Rim to	grade	М
Down	UNI	<	I	Rim to i	nvert		Grade to	invert	Rim to	grade	М
Use S	Sanita	ry	Direct	i <b>on</b> Dov	vnstream	Flo	w control		Medi	a No	
Shape	Circ	ular	Heig	ht 200	Width	mm	n Pred	clean J	Date Cle	aned 2023-02	2-27
Materia	al V	itrified Clay Pip	e	Joint	length	М	Total leng	gth 41.3 M	Length	Surveyed 41	.30 <b>M</b>
Lining				Ye	ar laid	Ye	ar rehabili	tated	Weather	Dry	
Purpos	se				C	Cat					
Additic	onal i	nfo						Structural	O & M	Constructi	onal
Locatio	on							Miscellaneous	Hydraulic		
Project	t	Lloydminster Pł	ase 1					Wor	k Order		
Northir	ng				Easting	3		Ele	vation		
Coordi	nate	System						GPS Accura	icy		





### Tabular Report of PSR UNK-527

for ISL

Tubului	nopo		01111 021			101							
Setup	4	Surveyo	r Ben Cooper	Certifica	ate #	U-315-0	06023	3838	S	System	Owner		
Drainag	je		Survey Custo	mer ISL									
P/O #			Date 2023-02-27	Time 16	5:00	Sti	eet 4	19th A	ve				
City	Lloyo	dminster	Further	location de	tails								
Up	UNK		Rin	n to invert		(	Grade	e to ir	ver	t	Rim t	o grade	М
Down	527		Rin	n to invert		(	Grade	e to ir	ver	t	Rim t	o grade	М
Use Sa	anitary		Direction	<b>n</b> Down		Flow	cont	rol			Ме	dia No	
Shape	Circular	r	Height	200 Widtl	h	mm		Precl	ean	J	Date C	leaned 2023-02-	-27
Material	l Vitrifi	ed Clay Pip	9	Joint lengtl	h	M 1	otal	lengt	<b>h</b> 55	.6 <b>M</b>	Lengt	h Surveyed 55.6	6 <b>M</b>
Lining				Year laid		Year	<sup>.</sup> reha	abilita	ted		Weathe	<b>r</b> Dry	
Purpose	e				Cat							Pressure	
Addition	nal info	)						S	tructu	ural	O & M	Constructio	nal
Locatio	n							N	iscel	laneous			
Project	Lloy	/dminster Pl	nase 1					L		Wo	rk Order		
Northing	g			Eas	sting					Ele	evation		
Coordin	nate Sy	stem			-				GPS	Accur	асу		
Count Vi	ideo	CD Code			In1	In2	%	.Int F	r To	ImRef	Romarks		
		ST	Start of Survey				/0				Remarks		
0.0		AM	H Manhole				_				UNK		
0.0		MW	L Water Level				10						
0.3		CM	Crack Multiple					1	2 12	!			
3.2		MW	L Water Level				25				25		
9.3		MW	L Water Level				10				10		
28.0		DAE	Deposits Attached End	crustation			15	0	3				
31.4		MW	L Water Level				25				25		
35.7		MW	L Water Level				15				15		

55.6 FH End of Survey

MWL

DAE Deposits Attached Encrustation

Water Level

AMH Manhole

55.6 M Total Length Surveyed

43.5

52.1

55.6

Scores	Structural:	Pipe Rating 3	Pipe Ratings Index 3	Peak 3	Mean Pipe 0.1
	O&M:	Pipe Rating 6	Pipe Ratings Index 3	Peak 3	Mean Pipe 0.1

15

40

03

40

527



#### Pipe Graphic Report of PSR UNK-527

for ISL

Setup	4	Surveyor	Ben Cooper	Cei	rtificate #	U-315	-06023838	System	Owner			
Drainage	!		Survey Cu	stomer	ISL							
P/O #			Date 2023-02-27	т	<b>ime</b> 16:00	S	treet 49th	n Ave				
City	Lloy	dminster	Furtl	her locat	ion details	5						
Up L	JNK			Rim to i	nvert		Grade to	o invert		Rim to g	rade	М
Down 5	27			Rim to i	nvert		Grade to	o invert		Rim to g	rade	м
Use San	itary		Direc	tion Dov	wnstream	Flo	w contro			Media	No	
Shape C	Circula	ır	Hei	<b>ght</b> 200	Width	mm	n Pre	eclean J	I	Date Clear	ned 2023-0	2-27
Material	Vitrif	fied Clay Pipe	•	Joint	t length	М	Total len	gth 55.6 M		Length S	urveyed 5	5.60 <b>M</b>
Lining				Ye	ar laid	Ye	ar rehabil	itated	V	Veather D	ry	
Purpose					C	at						
Additiona	al info	o						Structural	O 8	кM	Construct	ional
Location								Miscellaneou	<mark>is</mark> Hyc	draulic		
Project	Llo	ydminster Ph	ase 1				L	Wo	ork Ord	er		
Northing					Easting	J		E	levatior	า		
Coordina	ite Sy	vstem						GPS Accu	racy			



MCGIII'S Mc

### Tabular Report of PSR 527-512

for ISL

			021 012												
Setup	5	Surveyor	Ben Cooper	Certificate	<b>#</b> U-31	5-06023	3838	8	S	ystem	Owner				
Draina	age		Survey Custo	omer ISL											
P/O #			Date 2023-02-27	<b>Time</b> 16:14	1 ;	Street ·	49th	Ave	Э						
City	Lloye	dminster	Further	location detail	S										
Up	527		Riı	m to invert		Grad	e to	inv	ert		F	lim to	grade		М
Down	512		Rii	m to invert		Grad	e to	inv	vert		F	lim to	grade		м
Use	Sanitary		Directio	<b>n</b> Down	Flo	ow con	trol					Med	ia No		
Shape	Circula	r	Height	200 Width	m	n	Pre	clea	an .	J	Da	te Cle	aned 20	)23-02-2	27
Materi	i <b>al</b> Vitrif	ied Clay Pipe	U	Joint length	М	Total	lene	qth		м	Le	ength	Surveye	<b>ed</b> 101.	7 M
Lining				Year laid	Ye	ear reha	abili	itate	əd		We	ather	Dry		
Purpo	se			Ca	at								Pressu	re	
Additi	onal info	)						Stru	uctu	ral	O & N	1	Con	struction	al
Locati	ion							Mis	cell	aneous	6				
Projec	t Llo	vdminster Pha	ase 1							Wo	rk Order				
Northi	ina	,		Fastin	a					FI	evation				
Coord	linate Sv	stom		Eustin	9			G	PS						
		Stem								Accu	acy				
Count	Video	CD Code		In1	In2	2 %	Jnt	Fr	То	ImRe	f Remar	ks			
0.0		ST	Start of Survey								507				
0.0		AMH	Manhole			25					527				
0.0			Vvater Level			10		10							
0.0		DAG	Water Level	ease		25					25				
29.6			Joint Offset Medium												
43.7		DAF	Deposits Attached En	crustation		15		03	09						
63.3		MWL	Water Level			10					10				
74.5		DAE	Deposits Attached En	crustation		15		03	09						
80.7		CC	Crack Circumferential				J	09	05						
100.8		В	Broken					11	ĺ						İ

101.7 M Total Length Surveyed

MSA Abandoned Survey

101.7

Scores	Structural:	Pipe Rating 5	Pipe Ratings Index 1.7	Peak 3	Mean Pipe 0
	O&M:	Pipe Rating 8	Pipe Ratings Index 2.7	Peak 3	Mean Pipe 0.1

Drop insight, pipe damaged



#### Pipe Graphic Report of PSR 527-512

for ISL

Setup	į	5	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-0602383	8 Sys	tem Ow	vner		
Drainag	ge			Surv	ey Customer	ISL							
P/O #				Date 2023-	02-27	<b>Time</b> 16:14	5	Street 49	h Ave				
City	I	Lloyd	minster		Further loca	tion details	5						
Up	52	7			Rim to	invert		Grade t	o invert		Rim to	grade	М
Down	512	2			Rim to	invert		Grade t	o invert		Rim to	grade	М
Use Sa	anita	ary			Direction Do	wnstream	Flo	w contro	bl		Medi	a No	
Shape	Cir	cular			Height 200	Width	mn	n Pr	<b>eclean</b> J		Date Cle	aned 2023	-02-27
Materia	ı ı	Vitrifie	ed Clay Pipe	)	Join	t length	М	Total le	ngth	М	Length	Surveyed	101.70 <b>M</b>
Lining					Ye	ear laid	Ye	ar rehab	ilitated		Weather	Dry	
Purpos	е					C	Cat						
Additio	nal	info							Structur	al	O & M	Constru	ictional
Locatio	n								Miscella	neous	Hydraulic		
Project		Lloy	dminster Ph	ase 1						Work	Order		
Northin	g					Easting	J			Elev	ation		
Coordir	nate	e Sys	stem						GPS /	Accurac	;y		





CCTV Picture List of	527-512	for ISL	
Work Order			Setup 5
Video	Survey Date	2023-02-27	
Path to picture files	C:\FLEX6\Snaps\ISL\		
Path to video files	C:\FLEX6\Movies\ISL	Ν	
Path to media files	C:\FLEX6\Media\ISL\		
	eo Index Code Broken Remarks File Name 22.jpg	Count 100.8 M	



#### Tabular Report of PSR 896-897

for ISL

)	8	Surv	eyor	Ben Cooper	Certifi	cate #	U-315-	06023	3838		System	n Owner		
age			-	Survey Custo	mer ISL						-			
				Date 2023-02-28	Time	11:33	St	reet :	53rd /	Ave				
	Lloydn	ninster		Further	location o	details								
89	96			Rin	n to inver	t		Grad	e to i	nve	rt	Rim	to grade	М
89	)7			Rin	n to inver	t		Grad	e to i	nve	rt	Rim	to grade	М
Sanit	tary			Direction	<b>n</b> Down		Flov	v con	trol			Me	edia No	
e Ci	rcular			Height	300 Wid	dth	mm		Prec	lean	J	Date C	leaned 2023-02-	-28
ial	Vitrifie	d Clay	Pipe	· ·	Joint leng	qth	м	Total	leng	th 5	6.6 <b>M</b>	Lengt	h Surveyed 56.6	3 <b>M</b>
a		-			Year la	id	Yea	r reha	abilit	ated	I	Weathe	er Dry	
ose						Cat							Pressure	
iona	l info								5	Struc	tural	O & M	Constructio	nal
ion									N	/lisce	ellaneou	S		
ct	Lloyd	Iminste	er Phas	se 1							Wo	ork Order		
ing					Е	asting					E	levation		
dinat	e Syst	tem				U				GP	S Accu	racy		
Vida			odo			ln1	ln 2	0/	ln4	. т	a ImDo	f Domorko		
vide	<u>, o</u>		oue	Chart of Cumuou				70		<u> </u>				
				Start of Survey				_				896		
				Water Level				10						
				Water Level				20				20		
						1	4			0				
				Manhole	1			_				897		
			FH	End of Survey										
	age 88 88 Sani ial iona ct iing Uidat Vide	8 age Lloydn 896 897 Sanitary e Circular ial Vitrifie bse ional info ion ct Lloyc ing dinate Syst  Video	8   Survage     age   Lloydminster     896   897     Sanitary   e     e   Circular     ial   Vitrified Clay     g   Se     ional info   ion     ct   Lloydminster     ing   State     Video   CD     Image   Image     Video   CD     Image   Image     Image<	8   Surveyor     age   Lloydminster     896   897     Sanitary   e     e   Circular     ial   Vitrified Clay Pipe     g   See     ional info   ion     ct   Lloydminster Phase     ing   Strate System     Video   CD   Code     ST   AMH     MWL   TBI     AMH   FH	8   Surveyor Ben Cooper     age   Survey Custor     Date   2023-02-28     Lloydminster   Further     896   Rin     897   Rin     Sanitary   Direction     e   Circular   Height     ial   Vitrified Clay Pipe   Direction     9   See   See     ional info   See   See     ional info   Start of Survey     AMH   Manhole     MWL   Water Level     MWL   Water Level     TBI   Tap Break-in Intruding     AMH   Manhole     EH   End of Survey	8   Surveyor Ben Cooper   Certifiage     age   Survey Customer ISL     Date   2023-02-28     Lloydminster   Further location of     896   Rim to invertige     897   Rim to invertige     Sanitary   Direction Down     e   Circular     ial   Vitrified Clay Pipe     Joint leng   G     go   Year lai     ose   Start of Survey     Ing   E     dinate System   E     Video   CD Code     ST   Start of Survey     AMH   Manhole     MWL   Water Level     MWL   Water Level     TBI   Tap Break-in Intruding     AMH   Manhole     EH   End of Survey	8   Surveyor Ben Cooper   Certificate #     age   Survey Customer ISL   Date 2023-02-28   Time 11:33     Lloydminster   Further location details   896   Rim to invert     897   Rim to invert   897   Rim to invert     Sanitary   Direction Down   e   Circular   Height 300   Width     ial   Vitrified Clay Pipe   Joint length   g   Year laid     ose   Cat   Cat   Cat   Cat     ional info   Easting   Easting   Easting     dinate System   Start of Survey   In1   In1     ST   Start of Survey   In1   In1     MH   Manhole   Im1   Im1   Im1     MWL   Water Level   Im1   Im1   Im1     Im1   Tap Break-in Intruding   Im1   Im1   Im1   Im1	8   Surveyor Ben Cooper   Certificate # U-315-     age   Survey Customer ISL     Date 2023-02-28   Time 11:33   St     Lloydminster   Further location details     896   Rim to invert     897   Rim to invert     Sanitary   Direction Down     e Circular   Height 300     ial   Vitrified Clay Pipe   Joint length     M   Year     gose   Cat     ional info   Cat     ional info   Easting     dinate System   Start of Survey     Video   CD Code   In1     MWL   Water Level   In1     MWL   Water Level   In1     MWL   Water Level   In1     MWL   Math Manhole   In1     MH   Manhole   In1   In2	8   Surveyor Ben Cooper   Certificate # U-315-06023     age   Survey Customer ISL   Date 2023-02-28   Time 11:33   Street 8     Lloydminster   Further location details   896   Rim to invert   Grade     896   Rim to invert   Grade   897   Rim to invert   Grade     897   Rim to invert   Grade   897   Flow come     Sanitary   Direction Down   Flow come   Flow come     e Circular   Height 300   Width   mm     ial Vitrified Clay Pipe   Joint length   M   Total     ose   Cat   Cat   Sanitary   Vear rehation     ose   Cat   Cat   Sanitary   Sanitary   Sanitary     g   Year laid   Year rehation   Sanitary   Sanitary   Sanitary   Sanitary     g   Year laid   Year rehation   Sanitary   Sanit	8   Surveyor Ben Cooper   Certificate # U-315-06023838     age   Survey Customer ISL   Date 2023-02-28   Time 11:33   Street 53rd #     Lloydminster   Further location details   Street 53rd #   Grade to i     896   Rim to invert   Grade to i   6     897   Rim to invert   Grade to i   6     Sanitary   Direction Down   Flow control     e   Circular   Height 300   Width   mm   Preci     ial   Vitrified Clay Pipe   Joint length   M   Total length     g   Year laid   Year rehabilitation   Image: Street	8   Surveyor Ben Cooper   Certificate # U-315-06023838     age   Survey Customer ISL     Date 2023-02-28   Time 11:33   Street 53rd Ave     Lloydminster   Further location details   896     896   Rim to invert   Grade to inve     897   Rim to invert   Grade to inve     Sanitary   Direction Down   Flow control     e   Circular   Height 300   Width     ial   Vitrified Clay Pipe   Joint length   M   Total length 5     opse   Cat   Vear rehabilitated   Misce     ional info   Easting   Struce   Misce     total   Lloydminster Phase 1   In1   In2   % Jnt Fr     ig   ST   Start of Survey   In1   In2   % Jnt Fr     Video   CD   Code   In1   In2   % Jnt Fr   In1     MWL   Water Level   In1   In2   % Jnt Fr   In1   In1<	8   Surveyor Ben Cooper   Certificate # U-315-06023838   System age     age   Survey Customer ISL   Date 2023-02-28   Time 11:33   Street 53rd Ave     Lloydminster   Further location details   896   Rim to invert   Grade to invert     897   Rim to invert   Grade to invert   Grade to invert     Sanitary   Direction Down   Flow control     e   Circular   Height 300   Width   mm   Preclean J     ial   Vitrified Clay Pipe   Joint length   M   Total length 56.6   M     gse   Cat   Structural   Miscellaneou   Miscellaneou   Miscellaneou     ct   Lloydminster Phase 1   Wo   GPS Accu   Wo     ing   Easting   E   E   E     finate System   GPS Accu   Video   I   I   I     Video   CD Code   In1   In2   % JntFr To ImRe     MWL   Water Level   10   I   I   I     MWL   Water Level   20   I   I   I   I   I     MWL	8     Surveyor Ben Cooper     Certificate # U-315-06023838     System Owner       age     Survey Customer ISL     Date 2023-02-28     Time 11:33     Street 53rd Ave       Lloydminster     Further location details     Base     Rim to invert     Grade to invert     Rim       896     Rim to invert     Grade to invert     Rim     Rim     Street 53rd Ave       896     Rim to invert     Grade to invert     Rim     Rim     Street 53rd Ave       897     Rim to invert     Grade to invert     Rim     Rim     Street 53rd Ave       Sanitary     Direction Down     Flow control     Me     Me       9     Circular     Height 300     Width     mm     Preclean J     Date C       1al     Vitrified Clay Pipe     Joint length     M     Total length 56.6     M     Lengt       0     &     Year laid     Year rehabilitated     Weather       0     &     M     Miscellaneous     Miscellaneous       10     Logdminster Phase 1     Int     In2     %     M	8 Surveyor Ben Cooper Certificate # U-315-06023838 System Owner   age Survey Customer ISL Date 2023-02-28 Time 11:33 Street 53rd Ave   Lloydminster Further location details 896 Rim to invert Grade to invert Rim to grade   897 Rim to invert Grade to invert Rim to grade   Sanitary Direction Down Flow control Media No   a Circular Height 300 Width mm Preclean J Date Cleaned 2023-02-02-02-02-02-02-02-02-02-02-02-02-02-

56.6 M Total Length Surveyed

<b>Scores</b>	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 2	Pipe Ratings Index 2	Peak 2	Mean Pipe 0



# Pipe Graphic Report of PSR 896-897

for ISL

Setup		8	Surveyor	Ben Cooper	Cer	tificate #	U-315	06023838	System	Owner			
Drainag	ge		•	Survey C	ustomer	ISL			•				
P/O #				Date 2023-02-28	в <b>т</b>	ime 11:33	S	treet 53rd	Ave				
City		Lloyc	Iminster	Fur	ther locat	ion details	5						
Up	89	6			Rim to i	nvert		Grade to	o invert		Rim to g	grade	М
Down	89	7			Rim to i	nvert		Grade to	o invert		Rim to g	grade	М
Use Sa	anit	ary		Dire	ection Dov	vnstream	Flo	w contro			Media	a No	
Shape	Cir	cular		He	eight 300	Width	mm	Pre	clean J		Date Clea	ned 2023-	02-28
Materia	ıl	Vitrifi	ed Clay Pipe		Joint	length	М	Total len	gth 56.6 M		Length S	Surveyed 5	6.60 <b>M</b>
Lining					Yea	ar laid	Yea	ar rehabil	itated	١	Neather [	Dry	
Purpos	e					C	at						
Additio	nal	info	)						Structural	0 8	λ.Μ	Construc	tional
Locatio	n								Miscellaneou	s Hy	draulic		
Project		Lloy	dminster Ph	ase 1				-	We	ork Ord	er		
Northin	ıg					Easting	I		E	levatio	n		
Coordir	nate	e Sy	stem						GPS Accu	racy			



### Tabular Report of PSR 897-886

for ISL

Setup	9	5	Surveyor	Ben Cooper	Certific	ate #	U-315-	06023	3838	3	Syster	n Owne	ər			
Draina	ge		-	Survey Cus	tomer ISL						-					
P/O #	-			Date 2023-02-28	Time 1	1:49	St	reet 5	53rd	Ave						
City	LI	oydmir	nster	Furthe	r location de	etails										
Up	897			R	im to invert			Grade	e to	inve	rt		Rim t	o grade		М
Down	996			R	im to invert			Grade	e to	inve	rt		Rim t	o grade		м
Use S	Sanita	У		Directi	on Down		Flov	v cont	rol				Me	dia No		
Shape	Circ	ular		Heigh	t 300 Widt	h	mm		Pred	clear	۱J		Date C	leaned 2	023-02-2	8
Materi	al V	trified	Clay Pipe	· ·	Joint lengt	h	М	Total	leng	gth 5	52.5 <b>M</b>		Lengt	h Survey	<b>ed</b> 52.5	м
Lining					Year laid	I	Yea	r reha	abili	, tateo	ł		Weathe	r Dry		
Purpo	se					Cat								Pressu	ire	
Additi	onal i	nfo								Struc	tural	0	& M	Con	struction	al
Locati	on									Misc	ellaneou	JS				
Projec	t I	loydm	inster Pha	se 1							W	ork Orc	ler			
Northi	ng	-			Ea	sting					E	levatio	n			
Coord	inate	Syste	m			-				GF	S Accu	uracy				
Count \	/ideo	C	D Code			In1	In2	%	.Int	Fr 1	o ImR	ef Rer	narks			
0.0	1400		ST	Start of Survey		1				<u> </u>						
0.0			AMH	Manhole								897				
0.0			MWL	Water Level		1		10								
14.1			MWL	Water Level				15				15				
49.2			MWL	Water Level				25				25				
52.5			AMH	Manhole								996				
52.5			FH	End of Survey												Ī

52.5 M Total Length Surveyed

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



# Pipe Graphic Report of PSR 897-886

for ISL

Setup		9	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-06023838	3 System C	wner		
Draina	ge			Survey Cus	tomer	ISL						
P/O #				Date 2023-02-28	Т	<b>ime</b> 11:49	S	treet 53r	d Ave			
City		Lloyc	Iminster	Furthe	er locat	tion details	5					
Up	89	7		F	lim to i	nvert		Grade to	o invert	Rim t	o grade	М
Down	99	6		F	lim to i	nvert		Grade to	o invert	Rim t	o grade	М
Use S	Sanit	ary		Direct	i <b>on</b> Dov	wnstream	Flo	w contro	I	Ме	dia No	
Shape	Ci	rcular		Heig	<b>ht</b> 300	Width	mm	n Pre	eclean J	Date Cl	eaned 2023	-02-28
Materia	al	Vitrifi	ed Clay Pipe		Join	t length	М	Total ler	ngth 52.5 M	Length	n Surveyed	52.50 <b>M</b>
Lining					Ye	ar laid	Ye	ar rehabi	litated	Weathe	<b>r</b> Dry	
Purpos	se					c	at					
Additio	onal	info	)						Structural	O & M	Constru	ictional
Locatio	on								Miscellaneous	Hydraulic		
Project	t	Lloy	dminster Pha	ase 1					Wo	rk Order		
Northir	ng					Easting	I		Ele	evation		
Coordi	nat	e Sy	stem						GPS Accur	асу		





### Tabular Report of PSR 996-889

ISL for

		-				-	-						
Setup 1	0 <b>Sur</b>	veyor	Ben Cooper	C	ertificate #	U-315-	-06023	3838	S	ystem	Owner		
Drainage			Survey Cu	stomer	ISL								
P/O #			Date 2023-02-28	т	ime 12:57	St	treet t	53rd A	ve				
City L	loydminste	er	Furth	er locat	ion details								
<b>Up</b> 996	3		I	Rim to i	nvert		Grade	e to ir	nvert		Rim to	o grade	М
<b>Down</b> 889	)		I	Rim to i	nvert		Grade	e to ir	nvert		Rim to	o grade	М
Use Sanita	ary		Direct	t <b>ion</b> Dov	vn	Flov	v con	rol			Мес	dia No	
Shape Cire	cular		Heig	jht 300	Width	mm		Precl	ean .	I	Date Cl	eaned 2023-02	-28
Material V	/itrified Clag	y Pipe		Joint	length	М	Total	lengt	<b>h</b> 53.	4 <b>M</b>	Length	Surveyed 53.	4 <b>M</b>
Lining				Yea	ar laid	Yea	r reha	bilita	ted		Weather	Dry	
Purpose					Cat							Pressure	
Additional	info							S	tructu	ral	O & M	Constructio	onal
Location								N	liscella	aneous	;		
Project	Lloydminst	ter Phas	se 1							Wo	rk Order		
Northing					Easting					Ele	evation		
Coordinate	System								GPS	Accur	асу		
Count Video	D CD C	Code			In1	In2	%	Jnt F	r To	ImRef	Remarks		
0.0		ST	Start of Survey										
0.0		AMH	Manhole								996		
0.0		MWL	Water Level				0						
46.3		MWL	Water Level				30				30		
53.4		AMH	Manhole								889		

53.4 **M Total Length Surveyed** 

FH

AMH Manhole

End of Survey

53.4

53.4

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



# Pipe Graphic Report of PSR 996-889

for ISL

Sotup	10	Survovor	Ben Cooper	Cortifi	cato #	11-315-0	0003838	Systom (	wpor			
Setup	10	Surveyor	Dell Coopei	Certin	cale #	0-315-0	0023030	System	Wilei			
Drainage			Survey Cust	omer ISL	-							
P/O #			Date 2023-02-28	Time	<b>e</b> 12:57	St	reet 53rd	Ave				
City	Lloyo	dminster	Furthe	location	details							
<b>Up</b> 99	96		Ri	m to inve	ert		Grade to	invert	Rim to	grade	М	
<b>Down</b> 889			Ri	Rim to invert			Grade to	invert	Rim to	М		
Use Sani	itary		Directio	on Downs	tream	Flow	v control		Мес	lia No		
Shape Circular			Heigh	t 300 W	/idth	mm Preclean J			Date Cleaned 2023-02-28			
Material Vitrified Clay Pipe			•	Joint length M			M Total length 53.4 M			Length Surveyed 53.40 M		
Lining				Year I	aid	Yea	r rehabili	tated	Weather	Dry		
Purpose					С	at						
Additiona	ıl info	)						Structural	O & M	Constru	ctional	
Location								Miscellaneous	Hydraulic			
Project	Llo	ydminster Ph	ase 1					Wo	rk Order			
Northing Eastin					Easting			Ele	evation			
Coordinate System						GPS Accuracy						





#### Tabular Report of PSR 889-857

ISL for

Setup	11	Surve	yor	Ben Cooper	Certific	cate #	U-315-	06023	3838	S	System	Owner			
Drainage				Survey Custor	ner ISL										
P/O #				Date 2023-02-28	Time 1	13:07	St	reet :	53rd A	ve					
City	Lloydi	minster		Further le	ocation d	etails									
<b>Up</b> 8	89			Rim	to invert			Grad	e to i	nvert	:	Rim	to grade	М	
Down 8	57			Rim	to invert			Grad	e to i	nvert	t	Rim	М		
Use San	itary			Direction	Down		Flov	v con	trol			Me	Media No		
Shape C	ircular			Height	300 Wid	th	mm		Prec	ean	J	Date C	leaned 2023-02	2-28	
Material	Vitrifie	ed Clay F	Pipe	J	oint leng	th	М	Total	leng	<b>h</b> 61	.0 <b>M</b>	Lengt	h Surveyed 61	.0 <b>M</b>	
Lining					Year laid Year rehabilitated				Weather Dry						
Purpose						Cat							Pressure		
Additiona	al info								S	truct	ural	O & M	Constructi	onal	
Location									N	liscel	laneous	1			
Project	Lloy	dminster	Phas	se 1							Wo	rk Order			
Northing					Ea	sting					Ele	evation			
Coordina	te Sys	stem								GPS	Accur	асу			
Count Vid	ео	CD Co	de			In1	In2	%	JntF	r To	ImRef	Remarks			
0.0		S	Т	Start of Survey											
0.0		A	MH	Manhole								889			
0.0		N	1WL	Water Level				20							
23.3		D	AE	Deposits Attached Enci	ustation			20	C	)3					
61.0		A	MH	Manhole								857			

61.0 **M Total Length Surveyed** 

FH

AMH Manhole

End of Survey

61.0

61.0

Scores	Structural: Pipe Rating 0		Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 3	Pipe Ratings Index 3	Peak 3	Mean Pipe 0



# Pipe Graphic Report of PSR 889-857

for ISL

Setup	11	Surveyor	Ben Cooper	Certificat	e# U	J-315-0602	23838	System Ov	vner			
Drainage		-	Survey Cust	omer ISL				-				
P/O #			Date 2023-02-28	Time 1	3:07	Street	t 53rd Av	e				
City	Lloyo	dminster	Furthe	r location de	etails							
<b>Up</b> 88	39		Ri	m to invert		Gra	de to in	vert	Rim to	grade	М	
<b>Down</b> 857			Ri	Rim to invert			de to in	vert	Rim to	М		
<b>Use</b> Sani	tary		Directio	on Downstrea	am	Flow co	ntrol		Medi	ia No		
Shape Circular			Heigh	t 300 Widt	h	mm	Precle	an J	Date Cle	Date Cleaned 2023-02-28		
Material Vitrified Clay Pipe			)	Joint length			M Total length 61.0 M			Length Surveyed 61.00 M		
Lining				Year laid			habilitat	ed	Weather	Weather Dry		
Purpose					Cat							
Additiona	l info	)					Sti	uctural	O & M	Constru	ctional	
Location							Mi	scellaneous	Hydraulic			
Project Lloydminster Phase 1						Work	Order					
Northing Easting							Elev	vation				
Coordinate System							C	SPS Accura	су			



Phone:306-664-2220

INDUSTRIAL SERVICES

### Tabular Report of PSR 691-690

for ISL

Setup	27	Survey	or Ben Coope	r <b>C</b>	ertifica	ate #	U-315-	06023	3838		System	Owner			
Draina	age		Su	rvey Customer	ISL										
P/O #			Date 202	3-03-02	Time 13	3:08	St	reet 4	46th A	ve					
City Lloydminster Further location details															
Up	691			Rim to i	invert			Grade	e to i	nvei	rt	Rim to grade			Л
Down	690			Rim to i	invert			Grade	e to i	nvei	rt	Rim	to grade	Ν	Л
Use	Sanitary			Direction Do	wn		Flov	v cont	trol			Media No			
Shape	e Circula	r		Height 750	Widt	h	mm		Prec	ean	J	Date Cleaned			
Mater	ial Con	crete Pipe (	non-reinforced	) Join	t lengt	h	м	Total	leng	<b>h</b> 10	03.1 <b>M</b>	Lend	th Surveyed 1	03.1	м
Lining	1			Ye	ar laid		Yea	r reha	abilita	ated		Weath	Weather Dry		
Purpose Cat Pressure															
Additional info									S	truc	tural	O & M	Construc	tional	
Locat	ion								Ν	lisce	llaneou	3			
Projec	•t 1.10	udminster F	Phase 1								Wo	rk Order			
North	ina Elo	yammotor r			Fac	tina					FI	evation			
Coord	linato Sv	stom			Las	sung				CD					
COOID	iniale Sy	Stem								GF	5 ACCU	lacy			
Count	Video	CD Cod	е			In1	In2	%	<b>JntF</b>	r T	o ImRe	f Remarks			
0.0		ST	Start of Su	irvey											
0.0		AN	1H Manhole									691			
0.0		MV	VL Water Lev	el				30							
15.1		SR	Surface R	oughness Increas	sed				1	2					Ī
23.7		MV	VL Water Lev	el				20				20			1
84.8		DA	E Deposits A	Attached Encrusta	ation			5	C	2					7
103.1		AN	1H Manhole									690			Î

103.1 M Total Length Surveyed

FH

End of Survey

103.1

Scores	Structural:	Pipe Rating 1	Pipe Ratings Index 1	Peak 1	Mean Pipe 0
	O&M:	Pipe Rating 2	Pipe Ratings Index 2	Peak 2	Mean Pipe 0


### Pipe Graphic Report of PSR 691-690

for ISL

Setup	2	27	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-06023838	System	Owner			
Draina	ge		•	Sur	vey Customer	ISL			•				
P/O #				Date 2023	-03-02	<b>Time</b> 13:08	S	street 46th	Ave				
City	I	Lloyd	minster		Further loca	tion details	5						
Up	69 <sup>.</sup>	1			Rim to	invert		Grade to	invert		Rim to	grade	М
Down	690	0			Rim to	invert		Grade to	invert		Rim to	grade	М
Use S	Sanita	ary			Direction Do	wnstream	Flo	w control			Media	a No	
Shape	Shape Circular				Height 750	Width	mn	n Pre	clean J		Date Clea	aned	
Materia	Material Concrete Pipe (non-reinforced			on-reinforced)	Join	t length	М	Total len	gth 103.1 M		Length S	Surveyed	103.10 <b>M</b>
Lining	Material Concrete Pipe (non-reinforced				Ye	ear laid	Ye	ar rehabili	tated	,	Weather	Dry	
Purpos	se					C	Cat						
Additio	onal	info							Structural	08	& M	Constru	ictional
Locatio	on								Miscellaneou	s Hy	draulic		
Project Lloydminster Phase 1				ase 1				_	Wo	ork Ord	ler		
Northing					Easting	J		E	levatio	n			
Coordi	nate	e Sys	stem						GPS Accu	racy			



Phone:306-664-2220

INDUSTRIAL SERVICES

#### Tabular Report of PSR 690-689

for ISL

Setup	28	Su	rveyor	Ben Cooper		Certific	ate #	U-315-0	0602	3838		System	Owner			
Draina	age			Surv	ey Custome	er ISL										
P/O #				Date 2023-	03-02	Time 1	13:29	Sti	reet 4	46th A	ve					
City	Llo	/dminst	er		Further loc	ation d	etails									
Up	690				Rim to	o invert		(	Grad	e to i	nvei	rt	Rim	to grade		М
Down	689				Rim to	o invert		(	Grad	e to i	nvei	rt	Rim	to grade	l	М
Use S	Sanitary				Direction D	own		Flow	con	trol			N	ledia No		
Shape	Circul	ar			Height 75	0 Wid	th	mm		Prec	lean	J	Date	Cleaned		
Materi	i <b>al</b> Cor	ncrete P	Pipe (non	-reinforced)	Joi	int leng	th	M 1	Total	leng	t <b>h</b> 10	05.6 <b>M</b>	Leng	th Surveyed	105.6	Μ
Lining	I				Y	ear laid	d	Year	reha	abilita	ated		Weath	ner Dry		
Purpo	se						Cat							Pressure	•	
Additi	onal in	o								S	Struc	tural	O & M	Constr	uctiona	d
Locati	ion									N	lisce	llaneous	5			
Projec	t Lle	oydmins	ster Phas	se 1								Wo	rk Order			
Northi	ing					Ea	sting					El	evation			
Coord	linate S	ystem									GP	S Accu	racy			
Count V	Video	CD	Code				In1	In2	%	<b>JntF</b>	r T	o ImRe	f Remarks			
0.0			ST	Start of Surv	/ey											
0.0			AMH	Manhole									690			
0.0			MWL	Water Level					20							
4.1			SRI	Surface Rou	ighness Incre	ased				1	2					
9.2			DAE	Deposits Att	ached Encrus	station			5		)2					
53.0			DAE	Deposits Att	ached Encrus	station			5		8					

105.6 M Total Length Surveyed

AMH

FH

Manhole

End of Survey

105.6

105.6

Scores	Structural:	Pipe Rating 1	Pipe Ratings Index 1	Peak 1	Mean Pipe 0
	O&M:	Pipe Rating 4	Pipe Ratings Index 2	Peak 2	Mean Pipe 0

689



### Pipe Graphic Report of PSR 690-689

for ISL

Setup	:	28	Surveyor	Ben Cooper	Ce	ertificate #	U-315	-06023838	System 0	Dwner			
Draina	ge			Surv	vey Custome	r ISL							
P/O #				Date 2023	-03-02	Time 13:29	S	street 46th	Ave				
City	I	Lloyc	dminster		Further loca	ation details	5						
Up	690	0			Rim to	invert		Grade to	invert		Rim to g	grade	М
Down	689	9			Rim to	invert		Grade to	invert		Rim to g	grade	М
Use S	Sanita	ary			Direction Do	ownstream	Flo	w control			Media	a No	
Shape	Shape Circular				Height 750	Width	mn	n Pre	clean J	I	Date Clea	aned	
Materia	Material Concrete Pipe (non-reinforced			on-reinforced)	Joir	nt length	М	Total len	gth 105.6 M		Length S	Surveyed	105.60 <b>M</b>
Lining	Material Concrete Pipe (non-reinforced				Y	ear laid	Ye	ar rehabil	itated	V	Veather [	Dry	
Purpos	se					C	Cat						
Additio	onal	info	)						Structural	0 8	кM	Constru	ictional
Locatio	on								Miscellaneous	s Hyd	draulic		
Project Lloydminster Phase 1									Wo	rk Ord	er		
Northing					Easting	J		El	evatior	า			
Coordi	inate	e Sy	stem						GPS Accur	acy			





#### Tabular Report of PSR 692-691

for ISL

		•																
Setup		29	Surv	eyor	Ben Cooper		Certi	ficate #	U-315	-06023	3838	S	System	Owner				
Draina	age				Surv	vey Custon	ner ISL											
P/O #					Date 2023-	03-02	Time	<b>a</b> 14:25	S	treet 4	46th A	ve						
City		Lloyd	minster			Further lo	ocation	details										
Up	69	92				Rim	to inve	ert		Grad	e to i	nvert	:		Rim to	o grade		М
Down	69	91				Rim	to inve	ert		Grad	e to i	nvert	:		Rim to	o grade		М
Use	Sani	tary				Direction	Up		Flo	v con	trol				Med	dia No		
Shape	e Ci	ircular				Height 7	750 W	ʻidth	mm		Prec	ean	J	D	ate Cl	eaned 202	23-03-02	2
Mater	ial	Conc	rete Pip	e (non	-reinforced)	J	oint ler	ngth	М	Total	leng	<b>h</b> 10	5.0 <b>M</b>	L	.ength	Surveye	<b>d</b> 105.0	М
Lining	J						Year l	aid	Yea	ar reha	abilita	ated		W	eather	r Dry		
Purpo	se							Cat								Pressur	е	
Additi	ona	l info									S	structu	ural	O & I	M	Const	ructiona	al
Locati	ion										N	liscel	laneous	6				
Projec	ct	Lloy	dminste	er Phas	se 1								Wo	rk Orde	r			
North	ing						I	Easting					El	evation				
Coord	linat	te Sys	stem									GPS	Accur	асу				
Count	Vide	90	CD C	ode				In1	In2	%	JntF	r To	ImRe	FRema	rks			
0.0				ST	Start of Sur	/ev												
0.0				AMH	Manhole									691				
0.0				MWL	Water Leve					25								
84.8				SRI	Surface Rou	ughness Inci	reased				1	2						
105.0				АМН	Manhole									692				

105.0 M Total Length Surveyed

FH

End of Survey

105.0

Scores	Structural:	Pipe Rating 1	Pipe Ratings Index 1	Peak 1	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



# Pipe Graphic Report of PSR 692-691

for ISL

Setup	2	9 Surv	evor	Ben Cooper	С	ertificate #	U-31	5-06023838	System O	wner		
Draina	ge			Sur	vey Custome	r ISL						
P/O #				Date 2023	-03-02	Time 14:25	:	Street 46th	Ave			
City	L	loydminste	er		Further loc	ation detail	S					
Up	692				Rim to	invert		Grade to	o invert	Rim	to grade	М
Down	691				Rim to	invert		Grade to	o invert	Rim	to grade	М
Use S	Sanita	ry			Direction U	pstream	Flo	ow control		Ме	edia No	
Shape	Shape Circular				Height 75	0 Width	mr	n Pre	clean J	Date C	leaned 2023	3-03-02
Materia	Material Concrete Pipe (non-reinforced			on-reinforced)	Joi	nt length	М	Total len	gth 105.0 M	Lengt	h Surveyed	105.00 <b>M</b>
Lining	Material Concrete Pipe (non-reinforcec Lining				١	'ear laid	Ye	ear rehabil	itated	Weathe	<b>ər</b> Dry	
Purpos	se					(	Cat					
Additio	onal i	nfo							Structural	O & M	Constru	uctional
Locatio	on								Miscellaneous	Hydraulic		
Project Lloydminster Phase 1				ase 1					Wor	k Order		
Northing					Easting	g		Ele	vation			
Coordi	Coordinate System								GPS Accura	асу		





### Tabular Report of PSR1641-1273

for ISL

		•													
Setup	)	22	Sur	veyor	Ben Cooper	C	ertificate #	U-315	-06023	3838		System	n Owner		
Drain	age				Survey	Customer	ISL								
P/O #					Date 2023-03	3-02 <b>1</b>	<b>ime</b> 9:18	S	treet 7	18th \$	Stree	et			
City		Lloyd	minste	er	F	urther loca	tion details								
Up	16	641				Rim to i	nvert		Grad	e to i	inve	rt	Rim	to grade	М
Down	12	273				Rim to i	nvert		Grad	e to i	inve	rt	Rim	to grade	М
Use	Sani	tary			D	irection Do	wn	Flo	w con	trol			Me	edia No	
Shap	e Ci	ircular				Height 450	Width	mn	n	Prec	lear	n J	Date C	leaned 2023-03-0	02
Mater	ial	Polyv	inyl Cł	nloride		Join	t length	М	Total	leng	th 8	8.1 <b>M</b>	Lengt	h Surveyed 88.1	М
Lining	g					Ye	ar laid	Ye	ar reha	abilit	atec	1	Weathe	er Dry	
Purpo	ose						Cat							Pressure	
Addit	iona	l info								5	Struc	tural	O & M	Construction	nal
Locat	ion									ſ	Visc	ellaneou	IS		
Proje	ct	Lloy	dminst	ter Pha	se 1							W	ork Order		
North	ing						Easting					E	levation		
Coord	dinat	te Sys	stem								GP	S Accu	iracy		
Count	Vide	90	CD C	Code			In1	In2	%	Jntl	Fr T	o ImRe	ef Remarks		
0.0				ST	Start of Survey	/									
0.0				AMH	Manhole								1641		
0.0				MWL	Water Level				10						
88.1				AMH	Manhole								1273		

88.1 M Total Length Surveyed

FH

End of Survey

88.1

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



## Pipe Graphic Report of PSR 1641-1273

for ISL

Setup	:	22	Surveyor	Ben Cooper	Ce	rtificate #	U-315-	0602383	8 Syster	n Owner			
Draina	ge			Surv	vey Customer	ISL							
P/O #				Date 2023-	-03-02 1	<b>ime</b> 9:18	S	treet 18t	h Street				
City	I	Lloyo	dminster		Further loca	tion details	5						
Up	16	41			Rim to i	nvert		Grade t	o invert		Rim to	grade	М
Down	12	73			Rim to i	nvert		Grade t	o invert		Rim to	grade	М
Use S	Sanit	ary			Direction Do	wnstream	Flo	w contro			Media	a No	
Shape	Shape Circular				Height 450	Width	mm	Pr	eclean J		Date Clea	aned 2023-	03-02
Materia	Material Polyvinyl Chloride			)	Join	t length	м	Total le	ngth 88.1 I	N	Length S	Surveyed 8	38.10 <b>M</b>
Lining					Ye	ar laid	Yea	ar rehabi	litated	,	Weather	Dry	
Purpos	se					C	Cat						
Additio	onal	info	)						Structural	0	& M	Construe	ctional
Locatio	on								Miscellane	ous Hy	draulic		
Project	t	Lloy	ydminster Pha	ase 1					١	Nork Ord	ler		
Northing					Easting	3			Elevatio	n			
Coordinate System									GPS Ac	curacy			





### Tabular Report of PSR 1273-1642

for ISL

										_					
Setup	)	23	Sur	veyor	Ben Cooper	С	ertificate #	U-315	5-06023	3838	S	System	Owner		
Drain	age				Surve	y Customer	ISL								
P/O #					Date 2023-03	3-02 <b>T</b>	<b>ime</b> 9:34	S	Street	18th S	treet				
City		Lloyd	Iminste	er	F	Further locat	ion details								
Up	1:	273				Rim to i	nvert		Grad	e to i	nver	t	Rim t	o grade	М
Down	1	642				Rim to i	nvert		Grad	e to i	nver	t	Rim t	o grade	М
Use	San	itary			C	Direction Dov	vn	Flo	w con	trol			Ме	dia No	
Shape	e C	ircular				Height 450	Width	mm	า	Precl	ean	J	Date C	leaned 2023-03-	-02
Mater	ial	Polyv	vinyl Cł	nloride		Joint	length	М	Total	lengt	<b>h</b> 51	.9 <b>M</b>	Lengt	h Surveyed 51.9	9 <b>M</b>
Lining	g					Ye	ar laid	Ye	ar reha	abilita	ated		Weathe	<b>r</b> Dry	
Purpo	ose						Cat							Pressure	
Addit	iona	al info								S	truct	ural	O & M	Constructio	nal
Locat	ion									N	liscel	laneous	;		
Proje	ct	Lloy	dminst	ter Pha	se 1							Wo	rk Order		
North	ing						Easting					Ele	evation		
Coord	dina	te Sys	stem								GPS	6 Accur	асу		
Count	Vide	eo	CD (	Code			In1	In2	%	<b>Jnt</b> F	r To	ImRef	Remarks		
0.0				ST	Start of Surve	у									
0.0				AMH	Manhole								1273		
0.0				MWL	Water Level				10						
51.9			Τ	AMH	Manhole								1642		

51.9 M Total Length Surveyed

FH

End of Survey

51.9

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



## Pipe Graphic Report of PSR 1273-1642

for ISL

Setup	23	Surveyor	Ben Cooper	Cer	tificate #	U-315-	06023838	System O	wner			
Drainage			Survey	y Customer	ISL							
P/O #			Date 2023-03	3-02 <b>T</b>	<b>ime</b> 9:34	St	t <b>reet</b> 18th	n Street				
City	Lloyo	dminster	F	urther locat	ion details	6						
<b>Up</b> 12	273			Rim to ir	nvert		Grade to	o invert	Rim to	grade	М	
Down 16	642			Rim to in	nvert		Grade to	o invert	Rim to	grade	М	
Use Sani	itary		D	irection Dow	vnstream	Flow	v contro		Мес	lia No		
Shape C	Shape Circular			Height 450	Width	mm	Pre	eclean J	Date Cleaned 2023-03-02			
Material	Material Polyvinyl Chloride			Joint	length	Μ	Total ler	gth 51.9 M	Length	Surveyed	51.90 <b>M</b>	
Lining				Yea	ar laid	Yea	r rehabi	itated	Weather	Dry		
Purpose					C	Cat						
Additiona	al info	)						Structural	O & M	Constru	ictional	
Location								Miscellaneous	Hydraulic			
Project	Llo	ydminster Ph	ase 1					Wor	k Order			
Northing	Northing				Easting	J	Elevation					
Coordina	Coordinate System							GPS Accura	асу			





### Tabular Report of PSR 1642-1644

for ISL

			-				-	-						
Setup	24	Surve	eyor	Ben Cooper	C	ertificate #	U-315	-06023	3838	S	ystem	Owner		
Drainag	ge			Surve	y Customer	ISL								
P/O #				Date 2023-03	3-02 <b>T</b>	<b>`ime</b> 9:44	S	treet	18th S	treet				
City	Lloye	dminster		F	urther locat	ion details								
Up	1642				Rim to i	nvert		Grad	e to ir	nvert	:	Rim te	o grade	М
Down	1644				Rim to i	nvert		Grad	e to ir	nvert	:	Rim te	o grade	М
Use Sa	anitary			D	irection Dov	wn	Flo	w con	trol			Me	dia No	
Shape	Circula	r			Height 450	Width	mm	n	Precl	ean	J	Date CI	eaned 2023-0	3-02
Materia	l Poly	vinyl Chlo	oride		Join	t length	М	Total	lengt	h 77	.2 <b>M</b>	Length	n Surveyed 77	.2 <b>M</b>
Lining					Ye	ar laid	Ye	ar reha	abilita	ted		Weathe	<b>r</b> Dry	
Purpose	е					Cat							Pressure	
Additio	nal info	5							S	tructu	ıral	O & M	Constructi	onal
Locatio	n								N	liscel	laneous	3		
Project	Llo	ydminste	r Phas	se 1							Wo	rk Order		
Northin	g					Easting					El	evation		
Coordin	nate Sy	stem								GPS	Accu	acy		
Count Vi	ideo	CD Co	ode			In1	In2	%	<b>JntF</b>	r To	ImRe	f Remarks		
0.0		5	ST	Start of Surve	y									
0.0		A	AMH	Manhole								1642		
0.0		1	MWL	Water Level				15						
77.2		/	AMH	Manhole								1644		

77.2 M Total Length Surveyed

FH

End of Survey

77.2

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



## Pipe Graphic Report of PSR 1642-1644

for ISL

Setup	24	Surveyor	Ben Cooper	Cer	tificate #	U-315	-0602383	B System O	wner			
Drainage	•		Survey Cus	tomer	ISL							
P/O #			Date 2023-03-02	т	<b>'ime</b> 9:44	S	treet 18t	h Street				
City	Lloy	dminster	Furthe	er locat	ion details	6						
<b>Up</b> 1	642		F	lim to i	nvert		Grade t	o invert	Rim to	grade	М	
Down 1	644		F	lim to i	nvert		Grade t	o invert	Rim to	grade	М	
Use Sar	nitary		Direct	i <b>on</b> Dov	vnstream	Flo	w contro	I	Med	ia No		
Shape (	Shape Circular			<b>ht</b> 450	Width	mm	n Pro	eclean J	Date Cle	aned 2023-	03-02	
Material	Material Polyvinyl Chloride			Joint	length	М	Total ler	ngth 77.2 M	Length	Surveyed	77.20 <b>M</b>	
Lining				Ye	ar laid	Yea	ar rehabi	litated	Weather	Dry		
Purpose					C	Cat						
Addition	al infe	0						Structural	O & M	Constru	ctional	
Location								Miscellaneous	Hydraulic			
Project	Llo	ydminster Ph	ase 1					Wor	k Order			
Northing	Northing				Easting	)	Elevation					
Coordina	Coordinate System							GPS Accura	су			





### Tabular Report of PSR 1644-1643

for ISL

	-							-	-						
Setup	)	25	Sur	veyor	Ben Cooper	C	ertificate #	U-315	-06023	3838	S	ystem	Owner		
Drain	age				Surve	y Customer	ISL								
P/O #					Date 2023-03	3-02 <b>1</b>	<b>ime</b> 9:56	S	treet	18th S	Street				
City		Lloyd	minste	r	F	Further locat	ion details								
Up	16	644				Rim to i	nvert		Grad	e to i	nvert		Rim t	o grade	М
Down	16	643				Rim to i	nvert		Grad	e to i	nvert		Rim t	o grade	М
Use	Sani	itary			C	Direction Do	wn	Flo	w con	trol			Me	dia No	
Shape	e Ci	ircular				Height 450	Width	mm	n	Precl	ean	J	Date C	leaned 2023-03-	-02
Mater	ial	Polyv	inyl Cł	nloride		Join	t length	М	Total	lengt	<b>h</b> 53	.3 <b>M</b>	Lengt	n Surveyed 53.3	3 <b>M</b>
Lining	g					Ye	ar laid	Yea	ar reha	abilita	ated		Weathe	<b>r</b> Dry	
Purpo	ose						Cat							Pressure	
Addit	iona	ıl info								S	tructu	ıral	O & M	Constructio	nal
Locat	ion									N	liscel	laneous			
Proje	ct	Lloy	dminst	er Pha	se 1							Wor	k Order		
North	ing						Easting					Ele	evation		
Coord	dinat	te Sys	stem								GPS	Accura	асу		
Count	Vide	eo	CD C	Code			In1	In2	%	<b>JntF</b>	r To	ImRef	Remarks		
0.0				ST	Start of Surve	у									
0.0				AMH	Manhole								1644		
0.0				MWL	Water Level				15						
53.3				AMH	Manhole							·	1643		

53.3 M Total Length Surveyed

FH

End of Survey

53.3

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



## Pipe Graphic Report of PSR 1644-1643

for ISL

Setup	2	25	Surveyor	Ben Cooper	Cei	rtificate #	U-315	0602383	B System C	Owner		
Draina	ge			Surv	ey Customer	ISL						
P/O #				Date 2023-0	03-02 <b>T</b>	<b>ime</b> 9:56	S	treet 18t	h Street			
City	l	loyo	dminster		Further locat	ion details	5					
Up	164	44			Rim to i	nvert		Grade t	o invert	Rin	n to grade	М
Down	164	43			Rim to i	nvert		Grade t	o invert	Rin	n to grade	м
Use S	Sanita	ary			Direction Dov	wnstream	Flo	w contro		Ν	Media No	
Shape	Shape Circular				Height 450	Width	mm	Pr	eclean J	Date	Cleaned 20	23-03-02
Materia	al I	Poly	vinyl Chloride	•	Joint	t length	М	Total le	ngth 53.3 M	Leng	gth Surveye	<b>d</b> 53.30 <b>M</b>
Lining					Ye	ar laid	Yea	ar rehabi	litated	Weat	her Dry	
Purpos	se					C	Cat					
Additio	onal	info	)						Structural	O & M	Cons	tructional
Locatio	on								Miscellaneous	Hydrauli	С	
Project	t	Lloy	dminster Ph	ase 1					Wo	rk Order		
Northi	Northing					Easting	3		El	evation		
Coordi	Coordinate System								GPS Accur	асу		





#### Tabular Report of PSR 1643-838

54.6

54.6 **M** 

FH

**Total Length Surveyed** 

End of Survey

for ISL

		•														
Setup	)	26	Sur	veyor	Ben Cooper	C	ertificate #	U-315	-06023	3838		Syste	m Ow	ner		
Draina	age				Surve	y Customer	ISL									
P/O #					Date 2023-03	3-02 <b>1</b>	<b>'ime</b> 10:18	S	treet 1	8th S	Stree	et				
City		Lloyd	Iminste	ər	F	Further locat	ion details									
Up	16	643				Rim to i	nvert		Grade	e to i	nve	rt		Rim	to grade	М
Down	83	38				Rim to i	nvert		Grade	e to i	nve	rt		Rim	to grade	М
Use	San	itary			C	Direction Dov	wn	Flo	w con	rol				M	edia No	
Shape	e C	ircular				Height 450	Width	mm	n I	Prec	lean	J		Date 0	Cleaned 2023-03-0	02
Mater	ial	Polyv	vinyl C	hloride		Join	t length	М	Total	leng	t <b>h</b> 5	4.6 <b>M</b>	I	Leng	th Surveyed 54.6	M
Lining	9					Ye	ar laid	Yea	ar reha	bilit	ated	I		Weath	<b>er</b> Dry	
Purpo	se						Cat								Pressure	
Additi	iona	al info								5	Struc	tural	(	0 & M	Constructior	nal
Locat	ion									Ν	lisce	ellaneo	us			
Projec	ct	Lloy	dmins	ter Pha	se 1							W	lork O	rder		
North	ing						Easting					E	Elevat	ion		
Coord	lina	te Sys	stem								GP	S Acc	uracy			
Count	Vide	eo	CD	Code			In1	In2	%	JntF	r T	o ImR	ef R	emarks		
0.0				ST	Start of Surve	У										
0.0				AMH	Manhole								1643	3		
0.0				MWL	Water Level				15							
54.6				AMH	Manhole								838			

ScoresStructural:Pipe Rating 0Pipe Ratings Index 0Peak 0Mean Pipe 0O&M:Pipe Rating 0Pipe Ratings Index 0Peak 0Mean Pipe 0



## Pipe Graphic Report of PSR 1643-838

for ISL

Setup	:	26	Surveyor	Ben Cooper	Cer	tificate #	U-315-	0602383	B System C	Owner		
Draina	ge			Survey	Customer	ISL						
P/O #				Date 2023-03	-02 <b>T</b>	ime 10:18	S	treet 18t	h Street			
City	I	Lloyo	dminster	F	urther locat	ion details	5					
Up	16	43			Rim to i	nvert		Grade t	o invert	Ri	m to grade	М
Down	83	В			Rim to in	nvert		Grade t	o invert	Ri	m to grade	М
Use S	Sanit	ary		Di	irection Dov	vnstream	Flov	w contro	I		Media No	
Shape	Cir	cula	r	I	Height 450	Width	mm	Pre	eclean J	Date	e Cleaned 20	23-03-02
Materia	al	Poly	vinyl Chloride	•	Joint	length	М	Total ler	ngth 54.6 M	Ler	ngth Surveye	ed 54.60 M
Lining					Yea	ar laid	Yea	ar rehabi	litated	Weat	ther Dry	
Purpos	se					C	at					
Additio	onal	info	)						Structural	O & M	Cons	structional
Locati	on								Miscellaneous	Hydrau	lic	
Projec	t	Lloy	ydminster Pha	ase 1				·	Wo	rk Order		
Northi	ng					Easting	I		El	evation		
Coordi	Coordinate System								GPS Accur	acy		





### Tabular Report of PSR 1492-842

for ISL

		1.02 0.12									
Setup 6	Surveyor	Ben Cooper	Certificate	<b>#</b> U-315	-06023	3838	S	System	Owner		
Drainage		Survey Custo	omer ISL								
P/O #		Date 2023-02-28	<b>Time</b> 9:50	S	treet 2	23rd	Street				
City Llo	ydminster	Further	location detail	S							
<b>Up</b> 1492		Riı	m to invert		Grade	e to i	inver	t	Rim t	o grade	М
<b>Down</b> 842		Rii	m to invert		Grade	e to i	inver	t	Rim t	o grade	М
Use Sanitar	/	Directio	<b>n</b> Down	Flov	v cont	rol			Me	dia No	
Shape Circu	lar	Height	t 300 Width	mm	I	Prec	lean	J	Date Cl	leaned 2023-02-	-28
Material Po	lyvinyl Chloride	_	Joint length	М	Total	leng	<b>th</b> 98	.4 <b>M</b>	Length	n Surveyed 98.4	4 <b>M</b>
Lining			Year laid	Yea	ar reha	bilit	ated		Weathe	<b>r</b> Dry	
Purpose			Ca	t						Pressure	
Additional ir	fo					\$	Struct	ural	O & M	Constructio	nal
Location						1	Viscel	laneous	•		
Project L	loydminster Pha	ase 1						Wo	rk Order		
Northing			Eastin	g				Ele	evation		
Coordinate S	System			-			GPS	Accur	асу		
Count Video	CD Code		In1	In2	%	Jntl	Fr To	ImRef	Remarks		
0.0		Start of Survey									
0.0	AMH	Manhole							1492		
0.0	MWL	Water Level			10						
10.2	MWL	Water Level	İ		15				15		
22.9	MWL	Water Level			10				10		
61.2	MWL	Water Level			15				15		
72.3	MWL	Water Level			10				10		

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0

20

15

20

15

842



82.6

89.4

98.4

98.4

98.4 **M** 

MWL

MWL

AMH

FH

**Total Length Surveyed** 

Water Level

Water Level

End of Survey

Manhole

#### Pipe Graphic Report of PSR 1492-842

for ISL

		-	-		-							
Setup		6	Surveyor	Ben Cooper	Cer	tificate #	U-315-	06023838	System O	wner		
Draina	ge			Surve	ey Customer	ISL						
P/O #				Date 2023-0	)2-28 <b>T</b>	<b>ime</b> 9:50	S	t <b>reet</b> 23rd	Street			
City		Lloyo	dminster		Further locat	ion details	5					
Up	14	92			Rim to i	nvert		Grade to	invert	Rim to	grade	М
Down	84	2			Rim to in	nvert		Grade to	invert	Rim to	grade	М
Use S	Sanit	ary			Direction Dov	vnstream	Flov	v control		Med	lia No	
Shape	Ci	rcula	r		Height 300	Width	mm	Pre	clean J	Date Cle	aned 2023	-02-28
Materia	al	Poly	vinyl Chloride	9	Joint	length	м	Total leng	gth 98.4 M	Length	Surveyed	98.40 <b>M</b>
Lining					Ye	ar laid	Yea	r rehabili	tated	Weather	Dry	
Purpos	se					C	Cat					
Additio	onal	info	)						Structural	O & M	Constru	ictional
Locati	on								Miscellaneous	Hydraulic		
Projec	t	Lloy	ydminster Ph	ase 1				_	Worl	k Order		
Northi	Northing					Easting	9		Ele	vation		
Coordi	oortning Coordinate System								GPS Accura	су		





### Tabular Report of PSR 842-5420

for ISL

Setup	7	Surveyor	Ben Cooper	Certifica	ate # l	J-315-	06023	3838	;	System	Owner		
Draina	ige		Survey Cust	tomer ISL									
P/O #	•		Date 2023-02-28	Time 10	0:07	St	reet 2	23rd	Street	t			
City	Lloy	dminster	Furthe	r location de	tails								
Up	842		Ri	im to invert			Grade	e to	inver	t	Rim to	o grade	м
Down	5420		Ri	im to invert			Grade	e to	inver	t	Rim te	o grade	м
Use	Sanitarv		Directio	on Down		Flov	v cont	trol		-	Me	dia No	
Shape	Circula	r	Heigh	t 300 Widt	h	mm		Prec	lean	J	Date Cl	eaned 2023-02	-28
Materi	al Poly	vinvl Chloride		Joint lengt	h	м	Total	leng	th 91	.0 M	L ength	Surveyed 91.	 D M
Lining	u			Year laid	•	Yea	r reha	hilit	ated		Weather		,
Burno	<b>50</b>				Cat	100			aleu		Weather	Brossuro	
Purpo	se 				Gal							Flessure	
Additi	onal info	)							Struct	ural	0 & M	Constructio	nal
Locati	on								viisce	lianeous	5		
Projec	t Llo	ydminster Pha	ise 1							Wo	rk Order		
Northi	ng			Eas	sting					El	evation		
Coord	inate Sy	stem							GPS	S Accu	racy		
Count	Video	CD Code			In1	In2	%	Jnt	Fr To	o ImRe	f Remarks		
0.0		ST	Start of Survey										
0.0		AMH	Manhole								842		
0.0		MWL	Water Level				20						
13.6		MWL	Water Level				10				10		
26.7		MWL	S Water Level Sag				20				20		i
91.0		AMH	Manhole								5420		
91.0		FH	End of Survey										

91.0 M Total Length Surveyed

Scores	Structural:	Pipe Rating 2	Pipe Ratings Index 2	Peak 2	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



## Pipe Graphic Report of PSR 842-5420

for ISL

Setup		7	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-06023838	System O	wner		
Draina	ge			Survey Cu	stomer	ISL						
P/O #				Date 2023-02-28	г	<b>ime</b> 10:07	S	treet 23rd	d Street			
City		Lloy	dminster	Furth	ner locat	tion details	5					
Up	84	2			Rim to i	nvert		Grade to	o invert	Rim t	to grade	М
Down	54	20			Rim to i	nvert		Grade to	o invert	Rim t	to grade	М
Use S	Sanit	ary		Direc	tion Dov	wnstream	Flo	w contro	l	Ме	edia No	
Shape	Shape Circular				<b>ght</b> 300	Width	th mm Preclean J			Date C	leaned 2023	3-02-28
Materia	al	Poly	vinyl Chloride		Join	t length	М	Total ler	<b>191.0 M</b>	Lengt	h Surveyed	91.00 <b>M</b>
Lining					Ye	ar laid	Yea	ar rehabi	litated	Weathe	er Dry	
Purpos	se					c	at					
Additio	onal	info	)						Structural	O & M	Constru	uctional
Locatio	on								Miscellaneous	Hydraulic		
Project	t	Llo	ydminster Ph	ase 1				·	Wor	k Order		
Northi	Northing					Easting	J		Ele	vation		
Coordi	Coordinate System				GPS Accuracy							





## Tabular Report of PSR 1351-1352

FH

**Total Length Surveyed** 

End of Survey

for ISL

		•													
Setup	р	20	Sur	veyor	Ben Cooper	С	ertificate #	U-315	-06023	3838	S	System	Owner		
Drain	age				Survey	/ Customer	ISL								
P/O #	ŧ				Date 2023-03	-01 <b>T</b>	<b>ime</b> 17:34	S	treet	36th S	treet				
City		Lloyd	Iminste	er	F	urther locat	ion details								
Up	1	351				Rim to i	nvert		Grad	e to i	nvert		Rim t	o grade	М
Down	<b>n</b> 1	352				Rim to i	nvert		Grad	e to i	nvert	t	Rim t	o grade	М
Use	San	itary			D	irection Dov	vn	Flo	w con	trol			Ме	dia No	
Shap	<b>e</b> C	Circular				Height 525	Width	mm	1	Precl	ean	J	Date Cl	eaned	
Mater	rial	Polyv	vinyl Cł	nloride		Joint	length	М	Total	lengt	<b>h</b> 89	.7 <b>M</b>	Length	n Surveyed 89.3	7 <b>M</b>
Linin	g					Ye	ar laid	Yea	ar reha	abilita	ated		Weathe	<b>r</b> Dry	
Purpo	ose						Cat							Pressure	
Addit	tiona	al info								S	tructu	ural	O & M	Constructio	nal
Locat	tion									N	liscel	laneous	1		
Proje	ect	Lloy	dminst	ter Pha	se 1							Wo	rk Order		
North	ning						Easting					Ele	evation		
Coord	dina	te Sys	stem								GPS	Accur	асу		
Count	Vid	ео	CD (	Code			In1	In2	%	<b>JntF</b>	r To	ImRef	Remarks		
0.0				ST	Start of Survey	/									
0.0				AMH	Manhole								1351		
0.0				MWL	Water Level				10						
89.7				AMH	Manhole								1352		

 Scores
 Structural:
 Pipe Rating
 0
 Pipe Ratings
 Index
 0
 Peak
 0
 Mean
 Pipe
 0

 O&M:
 Pipe Rating
 0
 Pipe Ratings
 Index
 0
 Peak
 0
 Mean
 Pipe
 0



89.7

89.7 **M** 

## Pipe Graphic Report of PSR 1351-1352

for ISL

Setup	20	Surveyor	Ben Cooper	Cert	ificate #	U-315	-06023838	3 System	Owner				
Drainage			Survey Cus	tomer	SL								
P/O #			Date 2023-03-01	Ti	<b>me</b> 17:34	S	treet 36t	n Street					
City	Lloy	dminster	Furthe	r locati	on details	;							
<b>Up</b> 1	351		R	im to in	vert		Grade to	o invert		Rim to	grade	М	
Down 1	352		R	im to in	vert		Grade to	o invert		Rim to	grade	М	
Use San	itary		Directi	<b>on</b> Dow	nstream	Flo	w contro	I		Med	ia No		
Shape C	Circula	r	Heigl	nt 525	Width	mm	n Pre	eclean J		Date Cle	aned		
Material	Poly	vinyl Chloride	)	Joint	length	М	Total ler	ngth 89.7 M		Length	Surveyed	89.70	М
Lining				Yea	r laid	Yea	ar rehabi	litated	١	Neather	Dry		
Purpose					С	at							
Additiona	al info	)						Structural	0 8	λ.Μ	Constru	uctional	
Location								Miscellaneou	s Hy	draulic			
Project	Llo	ydminster Ph	ase 1					Wo	ork Ord	er			
Northing	lorthing East					I		E	levatio	n			
Coordina	Coordinate System					GPS Accuracy							





### Tabular Report of PSR 1352-1344

for ISL

		-														
Setup		21	Surv	veyor	Ben Cooper		Certificat	e # U-3	315-0	06023	3838	5	System	Owner		
Draina	age				Surve	ey Custom	er ISL									
P/O #					Date 2023-0	03-01	<b>Time</b> 17:	59	Str	eet 3	36th S	treet				
City		Lloydı	minste	r		Further lo	cation deta	ils								
Up	13	52				Rim t	o invert		C	Grade	e to ir	nver	t	Rim	to grade	Μ
Down	13	44				Rim t	o invert		C	Grade	e to ir	nver	t	Rim	to grade	М
Use	Sanit	ary				Direction	Down	F	Flow	cont	trol			M	edia No	
Shape	e Cir	cular				Height 5	25 Width	r	mm		Precl	ean	J	Date C	Cleaned	
Materi	ial	Polyvi	inyl Ch	loride		Jo	oint length	М	Т	otal	lengt	<b>h</b> 91	.5 <b>M</b>	Leng	th Surveyed 91.	5 <b>M</b>
Lining	J						Year laid		Year	reha	abilita	ted		Weath	<b>er</b> Dry	
Purpo	se						C	Cat							Pressure	
Additi	onal	info									S	truct	ural	O & M	Constructio	onal
Locati	ion										N	liscel	llaneou	5		
Projec	ct	Lloy	dminst	er Pha	se 1								Wo	rk Order		
Northi	ing						Easti	ng					EI	evation		
Coord	linate	e Sys	tem									GPS	6 Accu	racy		
Count	Vide	0	CD C	ode			In	1 I	n2	%	JntF	r To	ImRe	f Remarks		
0.0				ST	Start of Surve	еу										
0.0				AMH	Manhole									1352		
0.0				MWL	Water Level					10						
91.5				AMH	Manhole									1344		

91.5 M Total Length Surveyed

FH

End of Survey

91.5

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



## Pipe Graphic Report of PSR 1352-1344

for ISL

Setup	21	Surveyor	Ben Cooper	Certificate #	U-315-06023	838 System Ov	vner			
Drainage	e		Survey Custo	omer ISL						
P/O #			Date 2023-03-01	Time 17:59	Street	36th Street				
City	Lloy	dminster	Further	location details	6					
Up <sup>7</sup>	1352		Rir	n to invert	Grad	e to invert	Rim to	grade M		
Down <sup>~</sup>	1344		Rir	n to invert	Grad	e to invert	Rim to	grade M		
Use Sa	nitary		Directio	<b>n</b> Downstream	Flow con	trol	Medi	a No		
Shape (	Circula	ar	Height	525 Width	mm	Preclean J	Date Clea	aned		
Material	Poly	vinyl Chloride	)	Joint length	M Total	length 91.5 M	Length S	Surveyed 91.50 M		
Lining				Year laid	Year reha	abilitated	Weather	Dry		
Purpose	•			C	Cat					
Addition	al inf	0				Structural	O & M	Constructional		
Location	า					Miscellaneous	Hydraulic			
Project	Llo	ydminster Ph	ase 1			Work	Order			
Northing Ea					9	Elev	vation			
Coordinate System					GPS Accuracy					





### Tabular Report of PSR 1663-1662

for ISL

Setup	14	Survevor	Ben Cooper	Ce	ertificate #	U-315	-06023	3838	S	vstem	Owner		
Drainage			Survey Cust	omer	SL				-		-		
P/O #			Date 2023-02-28	Ti	<b>me</b> 19:05	S	treet 7	70th A	ve				
Citv	Llovdi	minster	Furthe	r locati	on details	•			-				
Un 16	662		Ri	im to in	vert		Grad	e to ir	wort		Rim to	arade	м
	002						Orau		IVCIL			grade	
Down 10	663		R	m to in	ivert		Grad	e to ir	vert		Rim to	o grade	М
Use Sani	itary		Directio	on Dow	'n	Flo	w con	rol			Med	dia No	
Shape C	ircular		Heigh	<b>t</b> 375	Width	mm		Precle	ean .	J	Date Cl	eaned 2023-0	2-28
Material	Polyv	inyl Chloride		Joint	length	М	Total	lengt	h 110	0.6 <b>M</b>	Length	Surveyed 1	10.6 <b>M</b>
Lining				Yea	r laid	Yea	ar reha	abilita	ted		Weather	<sup>.</sup> Dry	
Purpose					Cat							Pressure	
Additiona	al info							S	tructu	Iral	O & M	Construct	ional
Location								M	iscell	aneous	i		
Project	Lloy	dminster Pha	ase 1							Wo	rk Order		
Northing					Easting					Ele	evation		
Coordina	te Sys	tem			<b>J</b>				GPS	Accur	acy		
		CD Code			In4	In 2	0/	Int E	· To	ImDof	Bomarka		
							70	JILF	10	miker	Remarks		
0.0		ST	Start of Survey										
0.0		AMH	Manhole								1662		
0.0		MWL	Water Level				15						
110.6		AMH	Manhole								1663		

110.6 M Total Length Surveyed

FH

End of Survey

110.6

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



## Pipe Graphic Report of PSR 1663-1662

for ISL

Setup	14	Surveyor	Ben Cooper	Certificate	<b>#</b> U-31	5-06023838	System Ov	vner		
Drainage		-	Survey Custo	omer ISL			-			
P/O #			Date 2023-02-28	<b>Time</b> 19:	05	Street 70th	Ave			
City	Lloy	dminster	Further	location deta	ails					
<b>Up</b> 1/	662		Rii	n to invert		Grade to	invert	Rim to	grade	М
Down 1	663		Rii	n to invert		Grade to	invert	Rim to	grade	М
<b>Use</b> San	itary		Directio	n Downstream	FI	ow control		Medi	a No	
Shape C	Sircula	r	Height	375 Width	m	m Pre	clean J	Date Cle	aned 2023-	02-28
Material	Poly	vinyl Chloride		Joint length	м	Total leng	gth 110.6 M	Length	Surveyed <sup>·</sup>	10.60 <b>M</b>
Lining				Year laid	Y	ear rehabili	tated	Weather	Dry	
Purpose					Cat					
Additiona	al info	)					Structural	O & M	Construe	ctional
Location							Miscellaneous	Hydraulic		
Project	Llo	ydminster Ph	ase 1			_	Work	Order		
Northing				East	sting Elevation					
Coordina	ite Sy	stem			GPS Accuracy					





### Tabular Report of PSR 1662-1661

for ISL

		•													
Setup	c	15	Sur	veyor	Ben Cooper	С	ertificate #	U-315	-06023	3838	S	System	Owner		
Drain	age				Surve	y Customer	ISL								
P/O #	ŧ				Date 2023-03	3-01 <b>T</b>	ime 8:57	S	treet	70th A	ve				
City		Lloyd	Iminste	er	F	urther locat	tion details								
Up	1	662				Rim to i	nvert		Grad	e to i	nvert	t	Rim t	o grade	М
Down	<b>1</b> 1	661				Rim to i	nvert		Grad	e to i	nvert	t	Rim t	o grade	М
Use	San	itary			D	<b>irection</b> Dov	wn	Flo	w con	trol			Ме	dia No	
Shap	e C	Circular				Height 375	Width	mm	า	Prec	ean	J	Date C	leaned 2023-02	2-28
Mater	rial	Polyv	/inyl Cl	hloride		Join	t length	М	Total	lengt	<b>h</b> 89	.5 <b>M</b>	Lengtl	h Surveyed 89	.5 <b>M</b>
Linin	g					Ye	ar laid	Ye	ar reha	abilita	ated		Weathe	<b>r</b> Dry	
Purpo	ose						Cat							Pressure	
Addit	iona	al info	)							S	structu	ural	O & M	Construction	onal
Locat	tion									N	liscel	laneous			
Proje	ct	Lloy	/dmins	ter Pha	se 1							Woi	rk Order		
North	ning						Easting					Ele	evation		
Coord	dina	ite Sys	stem								GPS	Accur	асу		
Count	Vid	eo	CD (	Code			In1	In2	%	<b>Jnt</b> F	r To	ImRef	Remarks		
0.0				ST	Start of Surve	y									
0.0				AMH	Manhole								1662		
0.0				MWL	Water Level				10						
89.5				AMH	Manhole							$ $ $\top$	1661		

89.5 M Total Length Surveyed

FH

End of Survey

89.5

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0



## Pipe Graphic Report of PSR 1662-1661

for ISL

Setup	15	Surveyor	Ben Cooper	Cer	tificate #	U-315	06023838	System O	wner		
Drainage	•		Survey Cus	tomer	ISL						
P/O #			Date 2023-03-01	т	<b>ime</b> 8:57	S	treet 70th	n Ave			
City	Lloy	dminster	Furthe	r locat	ion details	5					
<b>Up</b> 1	662		R	im to i	nvert		Grade to	o invert	Rim to	grade	М
Down 1	661		R	im to i	nvert		Grade to	o invert	Rim to	grade	М
Use Sar	nitary		Directi	on Dov	vnstream	Flo	w control		Medi	ia No	
Shape (	Circula	ar	Heigh	<b>t</b> 375	Width	mm	Pre	clean J	Date Cle	aned 2023-0	02-28
Material	Poly	vinyl Chloride	)	Joint	length	М	Total len	gth 89.5 M	Length	Surveyed 8	9.50 <b>M</b>
Lining				Ye	ar laid	Yea	ar rehabil	itated	Weather	Dry	
Purpose					C	Cat					
Addition	al infe	0						Structural	O & M	Construc	tional
Location								Miscellaneous	Hydraulic		
Project	Llo	ydminster Ph	ase 1				-	Wor	k Order		
Northing					Easting	3		Ele	vation		
Coordinate System								GPS Accura	су		





#### Tabular Report of PSR 515-514

for ISL

rabala nope		•									
Setup 16	Surveyor Ben Coop	ber C	ertificate #	U-315-	06023	3838	Sy	stem	Owner		
Drainage	S	Survey Customer	ISL								
P/O #	Date 20	)23-03-01 <b>T</b>	ime 11:47	St	reet 3	85th Str	eet				
City Lloy	dminster	Further locat	ion details								
<b>Up</b> 515		Rim to i	nvert		Grade	e to in	/ert		Rim te	o grade	М
<b>Down</b> 514		Rim to i	nvert		Grade	e to inv	/ert		Rim te	o grade	М
Use Sanitary		Direction Dov	vn	Flow	v cont	rol			Ме	dia No	
Shape Circula	r	Height 375	Width	mm	l	Precle	an J		Date Cl	eaned 2023-03-	01
Material Vitrif	ied Clay Pipe	Joint	length	м	Γotal	length		М	Length	n Surveyed 66.0	) <b>M</b>
Lining		Ye	ar laid	Yea	r reha	bilitat	ed		Weathe	<b>r</b> Dry	
Purpose			Cat							Pressure	
Additional info	)					Str	uctur	al	O & M	Constructio	nal
Location						Mis	scella	neous			
Project Llo	dminster Phase 1							Wor	k Order		
Northing			Easting					Ele	evation		
Coordinate Sy	stem					G	PS /	Accura	асу		
Count Video	CD Code		In1	ln2	%	JntFr	To l	mRef	Remarks		
0.0	ST Start of S	Survey									
0.0	AMH Manhole	)						!	515		
0.0	MWL Water Le	evel			10						
17.4	MWL Water Le	evel			20			1	20		
18.8	DAE Deposits	s Attached Encrusta	tion		10	04					
20.3	DAE Deposits	s Attached Encrusta	tion		20	03					
35.8	DAE Deposits	s Attached Encrusta	tion		25	80					
44.8	DAGS Deposits	Attached Grease			5	02					
59.8	DAE Deposits	Attached Encrusta	tion		20	10					

66.0	м т.	401101		n vo vo d						
66.0			MSA	Abandoned Survey					DAE	
66.0			FC	Fracture Circumferential			12	04		
65.2			DAE	Deposits Attached Encrustation		35	03			
59.8			DAE	Deposits Attached Encrustation		20	10			

66.0 M Total Length Surveyed

Scores	Structural:	Pipe Rating 2	Pipe Ratings Index 2	Peak 2	Mean Pipe 0
	O&M:	Pipe Rating 19	Pipe Ratings Index 3.2	Peak 5	Mean Pipe 0.3



#### Pipe Graphic Report of PSR 515-514

for ISL

Setup	16/17 Surveyor	Ben Cooper	Certificate #	U-315-0602383	8 System Ov	wner	
Draina	ge	Survey Cust	omer ISL				
P/O #		Date 2023-03-01	<b>Time</b> 11:47	Street 35	th Street		
City	Lloydminster	Further	location details	6			
Up	515	Ri	m to invert	Grade t	o invert	Rim to	grade M
Down	514	Ri	m to invert	Grade t	o invert	Rim to	grade M
Use S	Sanitary	Directio	n Downstream	Flow contro	bl	Medi	a No
Shape	Circular	Height	t 375 Width	mm Pr	eclean J	Date Cle	aned 2023-03-01
Materia	I Vitrified Clay Pipe		Joint length	M Total le	ngth M	Length	Surveyed 66.00 M
Lining			Year laid	Year rehab	ilitated	Weather	Dry
Purpos	se		C	Cat			
Additic	onal info				Structural	O & M	Constructional
Locatio	on				Miscellaneous	Hydraulic	
Project	t Lloydminster Pha	ase 1			Work	Order	
Northir	ng		Easting	3	Elev	/ation	
Coordi	nate System				GPS Accura	су	





CCTV Picture List of	515-514	for ISL	
Work Order			Setup 16/17
Video	Survey Date	2023-03-01	
Path to picture files	C:\FLEX6\Snaps\ISL	λ	
Path to video files	C:\FLEX6\Movies\IS	L\	
Path to media files	C:\FLEX6\Media\ISL	Ν	
Vid	eo Index Code Deposits Attach Remarks File Name 34.jpg	Count 65.2 M ed Encrustation	



#### Tabular Report of PSR 515-514

for ISL

		•													
Setup	)	17	Sur	veyor	Ben Cooper	С	ertificate #	U-315	5-0602	3838	S	system	Owner		
Drain	age				Survey	Customer	ISL								
P/O #					Date 2023-03-	01 <b>T</b>	ime 12:07	S	Street	35th St	reet				
City		Lloyd	Iminste	ər	Fu	urther locat	ion details								
Up	5	15				Rim to i	nvert		Grad	e to in	vert		Rim	to grade	М
Down	5	14				Rim to i	nvert		Grad	e to in	vert		Rim	to grade	М
Use	San	itary			Di	rection Up		Flo	w con	trol			м	edia No	
Shape	e C	Circular			ŀ	Height 375	Width	mn	n	Precle	ean .	J	Date	Cleaned 2023-03-	-01
Mater	ial	Vitrifi	ed Cla	y Pipe		Joint	length	М	Total	lengtl	h	М	Leng	th Surveyed 18.4	4 <b>M</b>
Lining	9					Ye	ar laid	Ye	ar reh	abilita	ted		Weath	<b>er</b> Dry	
Purpo	se						Cat							Pressure	
Addit	iona	al info		Rev	erse set up on sh	eet:16				St	ructu	ural	O & M	Constructio	nal
Locat	ion									Μ	iscell	laneous	1		
Proje	ct	Lloy	dmins	ter Pha	se 1							Wo	rk Order		
North	ing						Easting					Ele	evation		
Coord	dina	te Sy	stem								GPS	Accur	асу		
Count	Vid	ео	CD (	Code			In1	In2	%	<b>JntF</b>	r To	ImRef	Remarks		
0.0				ST	Start of Survey										
0.0				AMH	Manhole								514		
1.0				MWL	Water Level				35						
18.4				DAE	Deposits Attach	ned Encrusta	tion		35	0	9				T

18.4 M Total Length Surveyed

MSA Abandoned Survey

18.4

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 5	Pipe Ratings Index 5	Peak 5	Mean Pipe 0.3

DAE



## Pipe Graphic Report of PSR 515-514

for ISL

Setup 17/16 Surveyor Ben Cooper	Certificate #	U-315-06023838	System Ow	ner	
Drainage Surv	ey Customer ISL				
P/O # Date 2023-0	<b>Time</b> 12:07	Street 35th S	Street		
City Lloydminster	Further location details	;			
<b>Up</b> 515	Rim to invert	Grade to i	nvert	Rim to	grade M
<b>Down</b> 514	Rim to invert	Grade to i	nvert	Rim to	grade M
Use Sanitary	Direction Upstream	Flow control		Media	a No
Shape Circular	Height 375 Width	mm Prec	lean J	Date Clea	aned 2023-03-01
Material Vitrified Clay Pipe	Joint length	M Total leng	th M	Length S	Surveyed 18.40 M
Lining	Year laid	Year rehabilit	ated	Weather	Dry
Purpose	C	at			
Additional info Reverse set up on	sheet:16	5	Structural	O & M	Constructional
Location		1	Viscellaneous	Hydraulic	
Project Lloydminster Phase 1			Work	Order	
Northing	Easting	I	Eleva	ation	
Coordinate System			GPS Accurac	у	





#### Tabular Report of PSR 514-513

for ISL

		-																		
Setu	р	18	Sur	veyor I	Ben Cooper	C	ertificat	e #	U-315-	06023	3838		S	ystem	Owner					
Drain	age	)			Survey Cus	tomer	ISL													
P/O #	ŧ				Date 2023-03-01	т	ime 13:	05	St	reet 3	35th \$	Stre	et							
City		Lloyd	Iminste	ər	Furthe	r locat	ion deta	ails												
Up	Ę	514			R	im to ir	nvert			Grade	e to	inve	ert		R	lim to	grade	,	М	1
Dowr	า 5	513			R	im to ir	nvert			Grade	e to i	inve	ert		R	lim to	grade	)	М	Í
Use	Sai	nitary			Directi	on Dov	vn		Flow	v con	trol					Med	lia No			
Shap	e (	Circular			Heigl	<b>1t</b> 375	Width		mm		Prec	lea	n J		Da	te Cle	aned	2023-03	3-01	
Mate	rial	Vitrifi	ed Cla	y Pipe	-	Joint	length		м	Total	leng	th		м	Le	ength	Surve	<b>yed</b> 82	.6	М
Linin	g					Yea	ar laid		Yea	r reha	abilit	ate	d		We	ather	Dry			
Purp	ose						(	Cat									Press	ure		
Addi	tion	al info										Stru	ctu	ral	O & M		Со	nstructi	onal	
Loca	tion											Viso	cella	aneous						
Proje	ct	Lloy	dmins	ter Phas	e 1									Wo	k Order					
North	nina						East	ina						Ele	vation					
Coor	dina	, ate Svs	stem					5				GI	PS	Accur	acv					
															,					
Count	Vic	leo	CD (	Code			Ir	า1	In2	%	Jntl	Fr	То	ImRef	Remar	ks				_
0.0				ST	Start of Survey															
0.0				AMH	Manhole										514					
0.0				MWL	Water Level					25										
5.8				DAE	Deposits Attached E	ncrusta	tion			20		04	80							1
7.5				DAGS	Deposits Attached G	Frease				10		02	11							1
33.6				OBZ	Obstacle Other					10		04	09		Rod					1
36.8				DAE	Deposits Attached E	ncrusta	tion			15		03						-		1
43.1				DAGS	Deposits Attached G	Frease				10		11						-		1
56.1				DAGS	Deposits Attached C	irease				10		01	11							1

82.6 M Total Length Surveyed

MSA Abandoned Survey

82.6

Scores	Structural:	Pipe Rating 0	Pipe Ratings Index 0	Peak 0	Mean Pipe 0
	O&M:	Pipe Rating 14	Pipe Ratings Index 2.3	Peak 3	Mean Pipe 0.2

manhole ahead



#### Pipe Graphic Report of PSR 514-513

for ISL

Setup	1	8	Surveyor	Ben Cooper	Cer	rtificate #	U-315	-0602383	8 Sys	tem Own	er			
Drainag	ge			Surv	ey Customer	ISL								
P/O #	P/O # Date 2023					1 Time 13:05 Street 35th Street								
City	L	loyo	dminster		Further location details									
<b>Up</b> 514					Rim to invert			Grade t	o invert		Rim to grade			
<b>Down</b> 513					Rim to i		Grade t	o invert		Rim to grade				
<b>Use</b> Sanitary					Direction Downstream Flow control				)		Med	Media No		
Shape Circular					Height 375	Width	mm Preclean J				Date Cleaned 2023-03-01			
Material Vitrified Clay Pipe				•	Joint length M Total length				ngth	М	Length	Surveyed	82.60	М
Lining				Year laid Year rehabilitated				litated		Weather	Dry			
Purpos	е					C	at							
Additio	nal	info	)						Structur	al (	D & M	Constr	uctional	
Locatio	n								Miscella	neous	Hydraulic			
Project		Lloy	/dminster Ph	ase 1						Work C	rder			
Northing					Easting					Elevation				
Coordinate System									GPS /	Accuracy				





### Tabular Report of PSR 513-512

for ISL

Setup	1	9	Surve	evor	Ben Cooper	Certific	ate #	U-315-	0602	3838	S	vstem	Owner			
Draina	qe				Survey Customer ISL											
P/O #	<b>J</b> -				Date 2023-03-01	Time 1	4:58	St	reet	35th S	treet					
City Llovdminster					Further	Further location details										
Up	513				Rir	n to invert			Grad	e to ir	vert		Rim f	to grade	м	
Down 512					Rir	Rim to invert			Grade to invert					Rim to grade		
Use S	Sanita	ry			Directio	Direction Down				trol			Ме	Media No		
Shape	Circ	ular			Height	Height 375 Width				Preck	ean .	J	Date Cleaned 2023-03-01			
Materia	al V	'itrifie	d Clav	Pipe		Joint lengt	h	M	Total	lenat	h 77.	9 <b>M</b>	Lenat	h Surveved 77	.9 <b>M</b>	
Linina			,			Year laid	1	Yea	r reh	abilita	ted		Weathe	er Drv		
Purpos	se					Cat						Pressure				
Additional info										S	tructu	ral	O & M	Constructio	onal	
Locati	on									М	iscell	aneous	3			
Projec	t	l lovd	Iminste	r Pha	se 1							Wo	rk Order			
Northi	- na	,.				Fa	stina					FI	evation			
Coordi	ing ingto	Svet	tom			Ea	Sting				CDS					
Coord	mate	Oysi										Accu	acy			
Count V	/ideo	<u> </u>	CD Co	ode			In1	In2		JntF	r To	ImRe	f Remarks			
0.0				ST	Start of Survey											
0.0			/	AMH	Manhole								513			
0.0			Ν	ЛWL	Water Level				25							
3.4			E	3	Broken					1	2					
26.0			1	ſBD	Tap Break-in Defective	e	4			0	2					
49.1			E	3	Broken					0	8 12					

77.9 M Total Length Surveyed

FH

AMH Manhole

End of Survey

77.9

77.9

Scores	Structural:	Pipe Rating 8	Pipe Ratings Index 4	Peak 5	Mean Pipe 0.1
	O&M:	Pipe Rating 3	Pipe Ratings Index 3	Peak 3	Mean Pipe 0

512



### Pipe Graphic Report of PSR 513-512

for ISL

Setup		19	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-0602383	B System C	)wner				
Draina	ge			Survey Cus	tomer	ISL								
P/O #				Date 2023-03-01	-	<b>Time</b> 14:58	S	Street 35t	h Street					
City		Lloy	dminster	Furthe	Further location details									
<b>Up</b> 513				R	Rim to invert			Grade t	o invert	Ri	Rim to grade			
<b>Down</b> 512				R	Rim to invert			Grade t	o invert	Ri	Rim to grade			
Use Sanitary				Directi	Direction Downstream Flow con			w contro	1		Media No			
Shape Circular				Heigh	nt 375	Width	mm Preclean J			Date	Date Cleaned 2023-03-0			
Material Vitrified Clay Pipe					Joint length			M Total length 77.9 M			Length Surveyed 77.90 M			
Lining					Year laid Year rehabilitated				litated	Weather Dry				
Purpos	se					C	Cat							
Additio	onal	info	)						Structural	O & M	Con	structional		
Locati	on								Miscellaneous	Hydrau	lic			
Projec	t	Llo	ydminster Ph	ase 1					Wo	rk Order				
Northi	ng				Easting					Elevation				
Coordi	inat	e Sy	stem						GPS Accur	асу				




CCTV Picture List of	513-512	for ISL	
Work Order			<b>Setup</b> 19
Video	Survey Date	2023-03-01	
Path to picture files	C:\FLEX6\Snaps\ISL	λ	
Path to video files	C:\FLEX6\Movies\ISI	L\	
Path to media files	C:\FLEX6\Media\ISL	Ι	
Vid	eo Index Code Tap Break-in De Remarks File Name 39.jpg	Count 26.0 M	



#### Tabular Report of PSR 613-624

for ISL

		•												
Setup	• 40	) <b>S</b>	Surveyor	Ben Cooper	Certif	icate #	U-315-	06023	3838	S	ystem	Owner		
Drain	age			Survey Custo	omer ISL									
P/O #				Date 2023-03-03	Time	14:01	St	reet $\mathfrak{t}$	52nd Av	/e				
City	LI	oydmin	ster	Further	location	details								
Up	613			Riı	n to inver	ť		Grade	e to in	vert		Rim t	М	
Down	614			Rim to invert Grade to in					e to in	o invert Rim to grade				М
Use	Sanitary Direction Down Flow control							trol			Ме	edia No		
Shape Circular Height 200						dth	mm		Precle	an .	J	Date C	leaned 2023-03-	03
Mater	rial ∨	itrified C	Clay Pipe		Joint len	gth	M	Total	length	75.	8 <b>M</b>	Lengt	h Surveyed 75.8	3 <b>M</b>
Linin	g				Year la	id	Yea	r reha	abilitat	ed		Weathe	er Dry	
Purpo	ose					Cat							Pressure	
Addit	ional i	nfo					St	uctu	Iral	O & M	Constructio	nal		
Locat	Location Miscellaneous													
Proje	ct	_loydmi	inster Pha	se 1					L		Wo	rk Order		
North	ing				E	asting					Ele	evation		
Coord	dinate	Syster	n			_			C	SPS	Accur	асу		
Count	Vidoo	-				In1	In2	0/	Int Er	То	ImPot	Pomarke		
	VIGEO			Start of Survey				/0				INCIII di KS		
0.0				Manhole								613		
0.0			MWL	Water Level				10						
12.8			MWL	Water Level				20				20		
13.5			OBM	Obstacle Pipe Materia	al			20	05					
46.4			Н	Hole					80					
60.1			DAE	Deposits Attached En	crustation			15	02					
62.9			DAE	Deposits Attached En	crustation			20	03					
75.8			AMH	Manhole								614		

75.8 M Total Length Surveyed

FH

End of Survey

75.8

Scores	Structural:	Pipe Rating 3	Pipe Ratings Index 3	Peak 3	Mean Pipe 0
	O&M:	Pipe Rating 9	Pipe Ratings Index 3	Peak 3	Mean Pipe 0.1



McGills Industrial Services Inc. Phone:306-664-2220

#### Pipe Graphic Report of PSR 613-624

INDUSTRIAL SERVICES

for ISL

Setup		40	Surveyor	Ben Cooper	Ce	rtificate #	U-315	-06023838	3 System (	Owner			
Draina	ge			Survey Cu	stomer	ISL			•				
P/O #				Date 2023-03-03	٦	<b>Time</b> 14:01	5	Street 52n	d Ave				
City		Lloyo	dminster	Furth	er loca	tion details	5						
Up	61	3		F	Rim to i	nvert		Grade to	o invert	R	im to grade	M	
Down	61	4		F	Rim to i	nvert		Grade to	o invert	R	im to grade	M	
Use S	Sanit	ary		Direct	ion Do	wnstream	Flo	w contro	I		Media No		
Shape	Ci	rcula	r	Heig	ht 200	Width	mn	n Pre	eclean J	Da	te Cleaned	2023-03-03	
Materi	al	Vitrif	ied Clay Pipe		Join	t length	М	Total ler	ngth 75.8 M	Le	ength Surve	<b>yed</b> 75.80	М
Lining					Ye	ar laid	Ye	ar rehabi	litated	We	ather Dry		
Purpo	se					c	Cat						
Additi	onal	info	)						Structural	O & M	Co	nstructional	
Locati	on								Miscellaneous	Hydra	ulic		
Projec	t	Llo	ydminster Ph	ase 1				L	Wo	rk Order			
Northi	ng					Easting	J		El	evation			
Coord	inat	e Sy	stem						GPS Accu	acy			



McGills Industrial Services Inc. Phone:306-664-2220







**APPENDIX** CCTV Inspection Videos (SharePoint Link)







APPENDIX Risk Assessment



#### Table G.1: Existing System Upgrades Risk Assessment Parameter Summary

				Category Weighted Score									
Upgrade No.	Name	Category	Length	Average Change in HGL	HGL Impact (2.4m)	HGL Impact (3.0m)	HGL Impact (3.5m)	HGL Impact (4.0m)	Existing Upstream Impact	Future Upstream Impact	Generalized Pipe Condition	Road Condition Upgrade Potential	Combined Weighted Score
1	Aurora Sewer Upgrades	Capacity	391.17	0.40	0.89	0.78	0.67	0.67	0.09	0.00	0.13	0.07	3.69
2	44 Street Sewer Upgrade	Capacity	273.47	0.40	0.00	0.16	0.27	0.27	0.27	0.20	0.13	0.02	1.71
3	51 Street Sewer Upgrade	Capacity	916.34	0.40	0.00	0.47	0.27	0.27	0.36	0.27	0.22	0.07	2.31
4	52 Street Trunk Upgrade	Capacity	807.62	0.60	0.00	0.00	0.13	0.13	0.36	0.27	0.22	0.04	1.76
5	48 Avenue Sewer Upgrade	Capacity	297.77	0.40	0.00	0.31	0.27	0.27	0.09	0.07	0.22	0.07	1.69
6	36 Street Trunk Upgrade West	Capacity	402.17	0.20	0.00	0.62	0.67	0.67	0.18	0.07	0.18	0.07	2.64
7	36 Street Trunk Upgrade East	Capacity	851.11	0.60	0.00	0.00	0.00	0.00	0.44	0.27	0.13	0.02	1.47
8	East Trunk Upgrade	Capacity	1060.35	0.40	0.00	0.00	0.00	0.00	0.44	0.33	0.13	0.11	1.42
9	Southeast Trunk Upgrade	Capacity	833.93	0.60	0.00	0.00	0.53	0.53	0.36	0.27	0.04	0.04	2.38
10	47 Avenue Sewer Upgrade	Capacity	234.14	0.80	0.00	0.31	0.40	0.40	0.09	0.00	0.18	0.07	2.24
11	53 Avenue Sewer Upgrade	Capacity	487.69	0.80	0.00	0.00	0.00	0.00	0.18	0.13	0.18	0.07	1.36
12	25 Street Trunk Upgrade	Capacity	386.00	1.00	0.00	0.00	0.00	0.00	0.09	0.13	0.13	0.02	1.38
13	59 Avenue Sewer Upgrade	Capacity	502.16	0.40	0.00	0.00	0.00	0.00	0.27	0.27	0.09	0.11	1.13
14	49 Avenue Sewer Replacement	Condition	100.93	0.00	0.00	0.00	0.00	0.00	0.09	0.00	0.22	0.04	0.36
15	42 Street East Sewer Replacement	Condition	71.02	0.00	0.00	0.00	0.00	0.00	0.09	0.00	0.18	0.04	0.31
16	42 Street West Sewer Replacement	Condition	358.06	0.00	0.00	0.00	0.00	0.00	0.09	0.00	0.18	0.07	0.33
17	36 Street Sewer Replacement	Condition	156.80	0.00	0.00	0.00	0.00	0.00	0.18	0.07	0.16	0.07	0.47
18	52 Avenue Sewer Replacement	Condition	148.15	0.00	0.00	0.00	0.00	0.00	0.09	0.00	0.18	0.09	0.36
19	46 Avenue Sewer Replacement	Condition	315.60	0.00	0.00	0.00	0.00	0.00	0.44	0.27	0.13	0.04	0.89
20	35 Street Sewer Replacement	Condition	164.92	0.00	0.00	0.00	0.00	0.00	0.09	0.00	0.20	0.07	0.36

#### SANITARY SEWER MASTER PLAN City of Lloydminster FINAL REPORT



#### Table G.2: Existing System Upgrades Risk Assessment Parameter Summary

			Average	HGL Imp	act (2.4m)	HGL Impa	act (3.0m)	HGL Impa	act (3.5m)	HGL Imp	oact (4.0m)	Existing Ups	tream Impact	Future Upstream Impact		Generalized	Road
Upgrade	Name	Category	Change in	Deserves	Denviation	Decomposito	Denulation	Description	Denulation	Description	Denulation	Catchment	Denviation	Catchment	Devulation	Pipe	Condition
NO.			m	Basements	Population	Basements	Population	Basements	Population	Basements	Population	ha	Population	ha	Population	Condition	Potential
1	Aurora Sewer Upgrades	Capacity	-3.66	63.00	262.40	63.00	262.40	63.00	262.40	71.00	295.72	7.03	562	0.00	0	Fair	Fair
2	44 Street Sewer Upgrade	Capacity	-3.76	0.00	0.00	2.00	0.00	6.00	22.28	10.00	25.07	295.35	5796	125.82	2125	Fair	Negligible
3	51 Street Sewer Upgrade	Capacity	-4.27	0.00	0.00	22.00	21.66	33.00	39.66	49.00	75.16	494.79	7846	373.79	3170	Failing	Fair
4	52 Street Trunk Upgrade	Capacity	-4.72	0.00	0.00	0.00	0.00	3.00	0.00	3.00	0.00	572.39	9840	373.79	3170	Failing	Poor
5	48 Avenue Sewer Upgrade	Capacity	-5.22	0.00	0.00	11.00	0.00	11.00	0.00	11.00	0.00	99.78	1833	0.19	0	Failing	Fair
6	36 Street Trunk Upgrade West	Capacity	-3.71	0.00	0.00	32.00	129.12	71.00	287.39	84.00	341.53	150.82	4435	0.34	0	Poor	Fair
7	36 Street Trunk Upgrade East	Capacity	-5.43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	220.85	11121	187.14	3733	Fair	Negligible
8	East Trunk Upgrade	Capacity	-4.48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	259.71	13023	1018.35	23883	Fair	Excellent
9	Southeast Trunk Upgrade	Capacity	-5.02	0.00	0.00	0.00	0.00	21.00	87.47	55.00	229.08	135.43	6628	186.15	3646	Excellent	Poor
10	47 Avenue Sewer Upgrade	Capacity	-5.87	0.00	0.00	6.00	24.99	19.00	79.14	19.00	79.14	18.97	1471	0.00	0	Poor	Fair
11	53 Avenue Sewer Upgrade	Capacity	-6.38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	124.01	3518	168.63	481	Poor	Fair
12	25 Street Trunk Upgrade	Capacity	-8.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	46.46	696	7.00	318	Fair	Negligible
13	59 Avenue Sewer Upgrade	Capacity	-4.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	261.94	4885	337.79	3170	Good	Excellent
14	49 Avenue Sewer Replacement	Condition	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.76	162	0.00	0	Failing	Poor
15	42 Street East Sewer Replacement	Condition	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	36.11	739	0.00	0	Poor	Poor
16	42 Street West Sewer Replacement	Condition	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	26.68	739	0.00	0	Poor	Fair
17	36 Street Sewer Replacement	Condition	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	95.88	3562	0.34	0	Poor/Fair	Fair
18	52 Avenue Sewer Replacement	Condition	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.94	157	0.00	0	Poor	Good
19	46 Avenue Sewer Replacement	Condition	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	288.56	12441	239.81	4739	Fair	Poor
20	35 Street Sewer Replacement	Condition	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	83.29	1958	0.00	0	Poor/Failing	Fair

#### SANITARY SEWER MASTER PLAN City of Lloydminster FINAL REPORT



City of Lloydminster FINAL REPORT



## Table G.3: Existing System Upgrades Risk Assessment - Net HGL Impact

Upgrade No.	Category	Maximum Change in HGL	Average Change in HGL	Raw Score	Weighted Score
		m	m		
1	Capacity	-5.09	-3.66	2	0.40
2	Capacity	-4.99	-3.76	2	0.40
3	Capacity	-5.03	-4.27	2	0.40
4	Capacity	-6.38	-4.72	3	0.60
5	Capacity	-5.34	-5.22	2	0.40
6	Capacity	-3.95	-3.71	1	0.20
7	Capacity	-6.00	-5.43	3	0.60
8	Capacity	-5.12	-4.48	2	0.40
9	Capacity	-5.96	-5.02	3	0.60
10	Capacity	-6.87	-5.87	4	0.80
11	Capacity	-6.91	-6.38	4	0.80
12	Capacity	-8.68	-8.10	5	1.00
13	Capacity	-5.15	-4.54	2	0.40
14	Condition	0.00	0.00	0	0.00
15	Condition	0.00	0.00	0	0.00
16	Condition	0.00	0.00	0	0.00
17	Condition	0.00	0.00	0	0.00
18	Condition	0.00	0.00	0	0.00
19	Condition	0.00	0.00	0	0.00
20	Condition	0.00	0.00	0	0.00



## SANITARY SEWER MASTER PLAN

City of Lloydminster FINAL REPORT

		HGL Im	pacted		Weighted
Upgrade No.	Category	Basement Count	Population	Raw Score	Score
1	Capacity	63	262.40	5	0.89
2	Capacity	0	0.00	0	0.00
3	Capacity	0	0.00	0	0.00
4	Capacity	0	0.00	0	0.00
5	Capacity	0	0.00	0	0.00
6	Capacity	0	0.00	0	0.00
7	Capacity	0	0.00	0	0.00
8	Capacity	0	0.00	0	0.00
9	Capacity	0	0.00	0	0.00
10	Capacity	0	0.00	0	0.00
11	Capacity	0	0.00	0	0.00
12	Capacity	0	0.00	0	0.00
13	Capacity	0	0.00	0	0.00
14	Condition	0	0.00	0	0.00
15	Condition	0	0.00	0	0.00
16	Condition	0	0.00	0	0.00
17	Condition	0	0.00	0	0.00
18	Condition	0	0.00	0	0.00
19	Condition	0	0.00	0	0.00
20	Condition	0	0.00	0	0.00

## Table G.4: Existing System Upgrades Risk Assessment - HGL Impact (2.4m)



## SANITARY SEWER MASTER PLAN

City of Lloydminster FINAL REPORT

		HGL I	mpact		Weighted
Upgrade No.	Category	Basement Count	Population	Raw Score	Score
1	Capacity	63	262.40	5	0.78
2	Capacity	2	0.00	1	0.16
3	Capacity	22	21.66	3	0.47
4	Capacity	0	0.00	0	0.00
5	Capacity	11	0.00	2	0.31
6	Capacity	32	129.12	4	0.62
7	Capacity	0	0.00	0	0.00
8	Capacity	0	0.00	0	0.00
9	Capacity	0	0.00	0	0.00
10	Capacity	6	24.99	2	0.31
11	Capacity	0	0.00	0	0.00
12	Capacity	0	0.00	0	0.00
13	Capacity	0	0.00	0	0.00
14	Condition	0	0.00	0	0.00
15	Condition	0	0.00	0	0.00
16	Condition	0	0.00	0	0.00
17	Condition	0	0.00	0	0.00
18	Condition	0	0.00	0	0.00
19	Condition	0	0.00	0	0.00
20	Condition	0	0.00	0	0.00

## Table G.5: Existing System Upgrades Risk Assessment - HGL Impact (3.0m)



City of Lloydminster FINAL REPORT



## Table G.6: Existing System Upgrades Risk Assessment - HGL Impact (3.5m)

	<b>O</b> oto warma	Potentially	HGL Impact		Weighted	
Upgrade No.	Category	Basement Count	Population	Raw Score	Score	
1	Capacity	63	262.40	5	0.67	
2	Capacity	6	22.28	2	0.27	
3	Capacity	33	39.66	2	0.27	
4	Capacity	3	0.00	1	0.13	
5	Capacity	11	0.00	2	0.27	
6	Capacity	71	287.39	5	0.67	
7	Capacity	0	0.00	0	0.00	
8	Capacity	0	0.00	0	0.00	
9	Capacity	21	87.47	4	0.53	
10	Capacity	19	79.14	3	0.40	
11	Capacity	0	0.00	0	0.00	
12	Capacity	0	0.00	0	0.00	
13	Capacity	0	0.00	0	0.00	
14	Condition	0	0.00	0	0.00	
15	Condition	0	0.00	0	0.00	
16	Condition	0	0.00	0	0.00	
17	Condition	0	0.00	0	0.00	
18	Condition	0	0.00	0	0.00	
19	Condition	0	0.00	0	0.00	
20	Condition	0	0.00	0	0.00	



## SANITARY SEWER MASTER PLAN

City of Lloydminster FINAL REPORT

## Table G.7: Existing System Upgrades Risk Assessment - HGL Impact (4.0m)

		Potentially	HGL Impact		Weighted	
Upgrade No.	Category	Basement Count	Population	Raw Score	Score	
1	Capacity	71	295.72	4	0.44	
2	Capacity	10	25.07	22.27968	2.48	
3	Capacity	49	75.16	3	0.33	
4	Capacity	3	0.00	1	0.11	
5	Capacity	11	0.00	2	0.22	
6	Capacity	84	341.53	5	0.56	
7	Capacity	0	0.00	0	0.00	
8	Capacity	0	0.00	0	0.00	
9	Capacity	55	229.08	4	0.44	
10	Capacity	19	79.14	4	0.44	
11	Capacity	0	0.00	0	0.00	
12	Capacity	0	0.00	0	0.00	
13	Capacity	0	0.00	0	0.00	
14	Condition	0	0.00	0	0.00	
15	Condition	0	0.00	0	0.00	
16	Condition	0	0.00	0	0.00	
17	Condition	0	0.00	0	0.00	
18	Condition	0	0.00	0	0.00	
19	Condition	0	0.00	0	0.00	
20	Condition	0	0.00	0	0.00	





		EXISU	ing opstream i	npaci			
Upgrade No.	Category	Catchment Area ha	Parcels	Population	Raw Score	Weighted Score	
1	Capacity	7.03	135	562	1	0.09	
2	Capacity	295.35	1,888	5,796	3	0.27	
3	Capacity	494.79	2,720	7,846	4	0.36	
4	Capacity	572.39	3,524	9,840	4	0.36	
5	Capacity	99.78	694	1,833	1	0.09	
6	Capacity	150.82	1,145	4,435	2	0.18	
7	Capacity	220.85	2,556	11,121	5	0.44	
8	Capacity	259.71	2,813	13,023	5	0.44	
9	Capacity	135.43	1,590	6,628	4	0.36	
10	Capacity	18.97	228	1,471	1	0.09	
11	Capacity	124.01	1,287	3,518	2	0.18	
12	Capacity	46.46	258	696	1	0.09	
13	Capacity	261.94	1,619	4,885	3	0.27	
14	Condition	2.76	41	162	1	0.09	
15	Condition	36.11	178	739	1	0.09	
16	Condition	26.68	145	739	1	0.09	
17	Condition	95.88	918	3562	2	0.18	
18	Condition	2.94	37	157	1	0.09	
19	Condition	288.56	3105	12441	5	0.44	
20	Condition	83.29	532	1958	1	0.09	

## Table G.8: Existing System Upgrades Risk Assessment - Existing Upstream Impact



#### SANITARY SEWER MASTER PLAN City of Lloydminster FINAL REPORT

		Future Upst	ream Impact		
Upgrade No.	Category	Catchment Area	Population	Raw Score	Weighted Score
		ha			
1	Capacity	0.00	0	0	0.00
2	Capacity	125.82	2,125	3	0.20
3	Capacity	373.79	3,170	4	0.27
4	Capacity	373.79	3,170	4	0.27
5	Capacity	0.19	0	1	0.07
6	Capacity	0.34	0	1	0.07
7	Capacity	187.14	3,733	4	0.27
8	Capacity	1,018.35	23,883	5	0.33
9	Capacity	186.15	3,646	4	0.27
10	Capacity	0.00	0	0	0.00
11	Capacity	168.63	481	2	0.13
12	Capacity	7.00	318	2	0.13
13	Capacity	337.79	3,170	4	0.27
14	Condition	0.00	0	0	0.00
15	Condition	0.00	0	0	0.00
16	Condition	0.00	0	0	0.00
17	Condition	0.34	0	1	0.07
18	Condition	0.00	0	0	0.00
19	Condition	239.81	4,739	4	0.27
20	Condition	0.00	0	0	0.00

#### Table G.9: Existing System Upgrades Risk Assessment - Future Upstream Impact



Upgrade No.	Category	Length	Year Installed	Material	Raw Score	Generalized Pipe Condition	Weighted Score
1	Capacity	391.17	1980-1986	VCT	3	Fair	0.13
2	Capacity	273.47	1987	VCT	3	Fair	0.13
3	Capacity	916.34	1948	VCT	5	Failing	0.22
4	Capacity	807.62	1948	VCT	5	Failing	0.22
5	Capacity	297.77	1940-1947	VCT	5	Failing	0.22
6	Capacity	402.17	1966	VCT	4	Poor	0.18
7	Capacity	851.11	1981	CONC	3	Fair	0.13
8	Capacity	1060.35	1981	CONC	3	Fair	0.13
9	Capacity	833.93	2006	PVC	1	Excellent	0.04
10	Capacity	234.14	1978	VCT	4	Poor	0.18
11	Capacity	487.69	1980-1986	VCT	4	Poor	0.18
12	Capacity	386.00	1988	CONC	3	Fair	0.13
13	Capacity	502.16	1982-1984	VCT/CONC	2	Good	0.09
14	Condition	100.93	1965	VCT	5	Failing	0.22
15	Condition	71.02	1990	CONC	4	Poor	0.18
16	Condition	358.06	1974	VCT	4	Poor	0.18
17	Condition	156.80	1966-1968	CONC/CMP	3.5	Poor/Fair	0.16
18	Condition	148.15	1973	VCT	4	Poor	0.18
19	Condition	315.60	1980	CONC	3	Fair	0.13
20	Condition	164.92	1973	VCT	4.5	Poor/Failing	0.20

#### Table G.10: Existing System Upgrades Risk Assessment - Generalized Pipe Condition



Upgrade No.	Category	Length	Imagery Year	Road Condition	Upgrade Coupling Potential	Road Condition Upgrade Potential	Raw Score	Weighted Score
1	Capacity	391.17	2009	Fair	Moderate	Fair	3	0.07
2	Capacity	273.47	2022	Good	Negligible	Negligible	1	0.02
3	Capacity	916.34	2012	Good/Fair	Moderate	Fair	3	0.07
4	Capacity	807.62	2012-2016	Fair	Low	Poor	2	0.04
5	Capacity	297.77	2012	Poor	Moderate	Fair	3	0.07
6	Capacity	402.17	2018	Fair/Poor	Moderate	Fair	3	0.07
7	Capacity	851.11	2018	Good/ Excellent	Negligible	Negligible	1	0.02
8	Capacity	1060.35	2022	Good/ Excellent	Very High	Excellent	5	0.11
9	Capacity	833.93	2009-2019	Good	Low	Poor	2	0.04
10	Capacity	234.14	2019	Fair	Moderate	Fair	3	0.07
11	Capacity	487.69	2012	Fair	Moderate	Fair	3	0.07
12	Capacity	386.00	2019	Good/ Excellent	Negligible	Negligible	1	0.02
13	Capacity	502.16	2019	Poor	Very High	Excellent	5	0.11
14	Condition	100.93	2009	Fair	Low	Poor	2	0.04
15	Condition	71.02	2019	Fair	Low	Poor	2	0.04
16	Condition	358.06	2012	Good/Fair	Moderate	Fair	3	0.07
17	Condition	156.80	2018	Fair	Moderate	Fair	3	0.07
18	Condition	148.15	2019	Poor	High	Good	4	0.09
19	Condition	315.60	2009	Good	Low	Poor	2	0.04
20	Condition	164.92	2012 and 2019	Fair	Some	Fair	3	0.07

## Table G.11: Existing System Upgrades Risk Assessment - Road Condition Upgrade Potential







APPENDIX Cost Estimates



pgrade No.	Item	Material	Quantity	Unit	Unit Cost <sup>1</sup>	Sub-Total	Contingency (30%)	Engineering (15%)	Total Cost
	250mm Gravity Sewer	PVC	240	Metres	\$560	\$134,571	\$40,371	\$26,241	\$210,000
	300mm Gravity Sewer	PVC	151	Metres	\$620	\$93,538	\$28,062	\$18,240	\$140,000
	200mm Gravity Sewer Removal	VCT	391	Metres	\$90	\$35,206	\$10,562	\$6,865	\$60,000
	Pavement Rehabilitation	N/A	391	Metres	\$1,030	\$402,909	\$120,873	\$78,567	\$610,000
		D) (O	070	Existing Upgra	de 1 Sub-Total:	\$666,223	\$199,867	\$129,914	\$1,020,000
	250mm Gravity Sewer	PVC	273	Metres	\$560	\$153,146	\$45,944	\$29,863	\$230,000
	200mm Gravity Sewer Removal	VCI	273	Metres	\$90	\$24,613	\$7,384	\$4,799	\$40,000
	Pavement Renabilitation	N/A	213	Interes	\$1,030	\$281,679	\$84,504	\$54,927	\$430,000
	275mm Crowity Sowor	BV/C	502	Existing Opgra		\$459,437	\$137,031	\$09,590	\$700,000
	300mm Gravity Sewer Removal	VCT	345	Metres	\$100	\$300,791	\$110,037	\$75,424	\$380,000
	Bail Crossing (Directional Drilling)	N/A	80	Metres	\$1.850	\$148,000	\$44,400	\$28,860	\$230,000
	Pavement Rehabilitation	N/A	345	Metres	\$1,000	\$354 927	\$106 478	\$69,211	\$540,000
			010	Existing Upgra	de 3 Sub-Total:	\$924,177	\$277.253	\$180.215	\$1.410.00
	300mm Gravity Sewer	PVC	15	Metres	\$620	\$9,338	\$2.801	\$1.821	\$20,000
	525mm Gravity Sewer	CONC	808	Metres	\$910	\$734,932	\$220,480	\$143,312	\$1,100,000
	450mm Gravity Sewer Removal	VCT	808	Metres	\$110	\$88,838	\$26,651	\$17,323	\$140,000
	Pavement Rehabilitation	N/A	823	Metres	\$1,030	\$847,359	\$254,208	\$165,235	\$1,270,000
				Existing Upgra	de 4 Sub-Total:	\$1,680,467	\$504,140	\$327,691	\$2,530,00
	375mm Gravity Sewer	PVC	298	Metres	\$770	\$229,286	\$68,786	\$44,711	\$350,000
	300mm Gravity Sewer Removal	VCT	276	Metres	\$100	\$27,566	\$8,270	\$5,375	\$50,000
	375mm Gravity Sewer Removal	PVC	22	Metres	\$100	\$2,211	\$663	\$431	\$10,000
	Rail Crossing	N/A	298	Metres	\$10,270	\$3,058,145	\$917,444	\$596,338	\$4,580,000
				Existing Upgra	de 5 Sub-Total:	\$3,317,209	\$995,163	\$646,856	\$4,990,000
	450mm Gravity Sewer	PVC	402	Metres	\$870	\$349,889	\$104,967	\$68,228	\$530,000
	375mm Gravity Sewer Removal	VCI	402	Metres	\$100	\$40,217	\$12,065	\$7,842	\$70,000
	Pavement Renabilitation	N/A	402	Metres	\$1,030	\$414,236	\$124,271	\$80,776	\$620,000
	000mm Cravity Source	CONC	051	Existing Upgra	de 6 Sub-Total:	\$804,342	\$241,303	\$155,847	\$1,220,000
	750mm Cravity Sewer		951	Metros	\$2,090 \$170	\$1,770,010 \$144,690	\$333,045 \$42,407	\$340,070 \$29,214	\$2,000,000
	Pavement Rehabilitation		42	Metres	\$170	\$42,009	\$43,407	\$8 376	\$220,000
		1.077	-12	Existing Upgra	de 7 Sub-Total:	\$1,966,461	\$589,938	\$383,460	\$2,950,000
	900mm Gravity Sewer	CONC	1.060	Metres	\$2,090	\$2,216,136	\$664,841	\$432,147	\$3,320,000
	750mm Gravity Sewer Removal	CONC	1.060	Metres	\$170	\$180.260	\$54.078	\$35,151	\$270.000
	Pavement Rehabilitation	N/A	336	Metres	\$1,030	\$346,563	\$103,969	\$67,580	\$520,000
				Existing Upgra	de 8 Sub-Total:	\$2,742,959	\$822,888	\$534,877	\$4,110,00
	525mm Gravity Sewer	PVC	834	Metres	\$910	\$758,875	\$227,663	\$147,981	\$1,140,000
a	375mm Gravity Sewer Removal	PVC	577	Metres	\$100	\$57,723	\$17,317	\$11,256	\$90,000
	450mm Gravity Sewer Removal	PVC	257	Metres	\$110	\$28,237	\$8,471	\$5,506	\$50,000
	Pavement Rehabilitation	N/A	834	Metres	\$1,030	\$858,947	\$257,684	\$167,495	\$1,290,000
				Existing Upgra	de 9 Sub-Total:	\$1,703,782	\$511,135	\$332,237	\$2,570,000
	300mm Gravity Sewer	PVC	234	Metres	\$620	\$145,169	\$43,551	\$28,308	\$220,000
	250mm Gravity Sewer Removal		234	Metres	\$90	\$21,073	\$6,322	\$4,109	\$40,000
		N/A	234	I Metres	\$1,030	\$241,168	\$72,350	\$47,028	\$370,000
	300mm Gravity Sewer	PV/C	263			\$407,409	\$122,223	\$79,445	\$530,000
	375mm Gravity Sewer	PVC	203	Metres	\$770	\$172 954	\$51 886	\$33,726	\$260,000
11	250mm Gravity Sewer Removal	VCT	263	Metres	\$90	\$23.676	\$7 103	\$4 617	\$40.000
	300mm Gravity Sewer Removal	VCT	225	Metres	\$100	\$22 462	\$6,738	\$4,380	\$40,000
	Pavement Rehabilitation	N/A	488	Metres	\$1,030	\$502.317	\$150.695	\$97.952	\$760.000
				Existing Upgrad	e 11 Sub-Total:	\$884.512	\$265.354	\$172.480	\$1,350.00
	375mm Gravity Sewer	PVC	386	Metres	\$770	\$297,221	\$89,166	\$57,958	\$450,000
12	300mm Gravity Sewer Removal	CONC	386	Metres	\$100	\$38,600	\$11,580	\$7,527	\$60,000
	Pavement Rehabilitation	N/A	386	Metres	\$1,030	\$397,582	\$119,275	\$77,528	\$600,000
				Existing Upgrad	e 12 Sub-Total:	\$733,403	\$220,021	\$143,014	\$1,110,00
	600mm Gravity Sewer	CONC	502	Metres	\$960	\$482,071	\$144,621	\$94,004	\$730,000
	450mm Gravity Sewer Removal	CONC	147	Metres	\$110	\$16,185	\$4,855	\$3,156	\$30,000
13	525mm Gravity Sewer Removal	CONC	355	Metres	\$120	\$42,603	\$12,781	\$8,308	\$70,000
	Pavement Rehabilitation	N/A	502	Metres	\$1,030	\$517,222	\$155,166	\$100,858	\$780,000
	Rail Crossing (Directional Drilling)	N/A	110	Metres	\$1,850	\$203,500	\$61,050	\$39,683	\$310,000
				Existing Upgrad	e 13 Sub-Total:	\$1,261,580	\$378,474	\$246,008	\$1,920,00



Upgrade No.	Item	Material	Quantity	Unit	Unit Cost <sup>1</sup>	Sub-Total	Contingency (30%)	Engineering (15%)	Total Cost <sup>2</sup>
	200mm Gravity Sewer	PVC	101	Metres	\$540	\$54,505	\$16,351	\$10,628	\$90,000
14	200mm Gravity Sewer Removal	VCT	101	Metres	\$90	\$9,084	\$2,725	\$1,771	\$20,000
	Pavement Rehabilitation	N/A	101	Metres	\$1,030	\$103,962	\$31,189	\$20,273	\$160,000
				Existing Upgrad	e 14 Sub-Total:	\$167,551	\$50,265	\$32,672	\$270,000
	375mm Gravity Sewer	PVC	71	Metres	\$770	\$54,682	\$16,404	\$10,663	\$90,000
15	375mm Gravity Sewer Removal	CONC	71	Metres	\$100	\$7,102	\$2,130	\$1,385	\$20,000
	Pavement Rehabilitation	N/A	71	Metres	\$1,030	\$73,146	\$21,944	\$14,263	\$110,000
				Existing Upgrad	e 15 Sub-Total:	\$134,929	\$40,479	\$26,311	\$220,000
	375mm Gravity Sewer	PVC	358	Metres	\$770	\$275,704	\$82,711	\$53,762	\$420,000
16	375mm Gravity Sewer Removal	VCT	358	Metres	\$100	\$35,806	\$10,742	\$6,982	\$60,000
	Pavement Rehabilitation	N/A	358	Metres	\$1,030	\$368,799	\$110,640	\$71,916	\$560,000
				Existing Upgrad	e 16 Sub-Total:	\$680,309	\$204,093	\$132,660	\$1,040,000
	375mm Gravity Sewer	PVC	157	Metres	\$770	\$120,734	\$36,220	\$23,543	\$190,000
17	375mm Gravity Sewer Removal	VCT	157	Metres	\$100	\$15,680	\$4,704	\$3,058	\$30,000
	Pavement Rehabilitation	N/A	157	Metres	\$1,030	\$161,501	\$48,450	\$31,493	\$250,000
				Existing Upgrad	e 17 Sub-Total:	\$297,914	\$89,374	\$58,093	\$470,000
	200mm Gravity Sewer	PVC	148	Metres	\$540	\$80,001	\$24,000	\$15,600	\$120,000
18	200mm Gravity Sewer Removal	CONC/CMP	148	Metres	\$90	\$13,333	\$4,000	\$2,600	\$20,000
	Pavement Rehabilitation	N/A	148	Metres	\$1,030	\$152,594	\$45,778	\$29,756	\$230,000
				Existing Upgrad	e 18 Sub-Total:	\$245,929	\$73,779	\$47,956	\$370,000
	750mm Gravity Sewer	CONC	316	Metres	\$1,350	\$426,064	\$127,819	\$83,082	\$640,000
19	750mm Gravity Sewer Removal	CONC	316	Metres	\$170	\$53,652	\$16,096	\$10,462	\$90,000
	Pavement Rehabilitation	N/A	316	Metres	\$1,030	\$325,071	\$97,521	\$63,389	\$490,000
				Existing Upgrad	e 19 Sub-Total:	\$804,787	\$241,436	\$156,933	\$1,220,000
	375mm Gravity Sewer	PVC	165	Metres	\$770	\$126,988	\$38,097	\$24,763	\$190,000
20	375mm Gravity Sewer Removal	VCT	165	Metres	\$100	\$16,492	\$4,948	\$3,216	\$30,000
	Pavement Rehabilitation	N/A	165	Metres	\$1,030	\$169,868	\$50,960	\$33,124	\$260,000
				Existing Upgrad	e 20 Sub-Total:	\$313,348	\$94,004	\$61,103	\$480,000
					Total:	\$2,650,000	\$800,000	\$520,000	\$4,070,000

<sup>1</sup> Costs herein are comparable to other municipalities. Costs are representative of 2024 dollars. Unit costs have been rounded to the nearest \$10,000. <sup>2</sup> The total cost has been rounded up to the nearest \$10,000.



# SANITARY SEWER MASTER PLAN City of Lloydminster FINAL REPORT

Table H.3: Future System Concept Cost Estimates

Upgrade No.	Item	Material	Quantity	Unit	Unit Cost <sup>1</sup>	Sub-Total	Contingency (30%)	Engineering (15%)	Total Cost <sup>2</sup>
	200mm Forcemain	PVC	916	Metres	\$540	\$494,424	\$148,327	\$96,413	\$740,000
	300mm Gravity Sewer	PVC	929	Metres	\$620	\$575,794	\$172,738	\$112,280	\$870,000
	375mm Gravity Sewer	PVC	1,347	Metres	\$770	\$1,037,421	\$311,226	\$202,297	\$1,560,000
	450mm Gravity Sewer	PVC	4,006	Metres	\$870	\$3,485,394	\$1,045,618	\$679,652	\$5,220,000
	525mm Gravity Sewer	PVC	848	Metres	\$910	\$771,771	\$231,531	\$150,495	\$1,160,000
Concept 1	600mm Gravity Sewer	PVC	3,097	Metres	\$960	\$2,973,504	\$892,051	\$579,833	\$4,450,000
Concept 1	675mm Gravity Sewer	PVC	1,888	Metres	\$1,160	\$2,189,964	\$656,989	\$427,043	\$3,280,000
	750mm Gravity Sewer	PVC	1,722	Metres	\$1,350	\$2,324,700	\$697,410	\$453,317	\$3,480,000
	900mm Gravity Sewer	PVC	993	Metres	\$2,090	\$2,074,534	\$622,360	\$404,534	\$3,110,000
	1050mm Gravity Sewer	PVC	1,292	Metres	\$2,250	\$2,906,325	\$871,898	\$566,733	\$4,350,000
	1200mm Gravity Sewer	CONC	7,445	Metres	\$2,400	\$17,868,000	\$5,360,400	\$3,484,260	\$26,720,000
	Lift Station 1 (31 L/s)	N/A	1	Item	\$700,000	\$700,000	\$210,000	\$136,500	\$1,050,000
				Future C	oncept 1 Total:	\$37,401,831	\$11,220,549	\$7,293,357	\$55,990,000
	200mm Forcemain	PVC	916	Metres	\$540	\$494,424	\$148,327	\$96,413	\$740,000
	300mm Gravity Sewer	PVC	929	Metres	\$620	\$575,794	\$172,738	\$112,280	\$870,000
	375mm Gravity Sewer	PVC	1,347	Metres	\$770	\$1,037,421	\$311,226	\$202,297	\$1,560,000
	450mm Gravity Sewer	PVC	4,278	Metres	\$870	\$3,721,773	\$1,116,532	\$725,746	\$5,570,000
	525mm Gravity Sewer	PVC	848	Metres	\$910	\$771,771	\$231,531	\$150,495	\$1,160,000
	600mm Gravity Sewer	PVC	3,653	Metres	\$960	\$3,506,784	\$1,052,035	\$683,823	\$5,250,000
Concept 2	675mm Gravity Sewer	PVC	792	Metres	\$1,160	\$918,720	\$275,616	\$179,150	\$1,380,000
	750mm Gravity Sewer	PVC	2,021	Metres	\$1,350	\$2,728,350	\$818,505	\$532,028	\$4,080,000
	900mm Gravity Sewer	PVC	961	Metres	\$2,090	\$2,008,490	\$602,547	\$391,656	\$3,010,000
	1050mm Gravity Sewer	PVC	829	Metres	\$2,250	\$1,865,250	\$559,575	\$363,724	\$2,790,000
	1200mm Gravity Sewer	CONC	7,128	Metres	\$2,400	\$17,107,200	\$5,132,160	\$3,335,904	\$25,580,000
	1350mm Gravity Sewer	PVC	1,639	Metres	\$2,560	\$4,195,840	\$1,258,752	\$818,189	\$6,280,000
	Lift Station 1 (31 L/s)	N/A	1	Item	\$700,000	\$700,000	\$210,000	\$136,500	\$1,050,000
				Future C	oncept 2 Total:	\$39,631,817	\$11,889,545	\$7,728,204	\$59,320,000

<sup>1</sup> Costs herein are comparable to other municipalities. Costs are representative of 2024 dollars. Unit costs have been rounded to the nearest \$10,000. <sup>2</sup> The total cost has been rounded up to the nearest \$10,000.



## SANITARY SEWER MASTER PLAN City of Lloydminster FINAL REPORT

Concept No.	Upgrade No.	Item	Material	Quantity	Unit	Unit Cost <sup>1</sup>	Sub-Total	Contingency (30%)	Engineering (15%)	Total Cost <sup>2</sup>
		900mm Gravity Sewer Removal	CONC	750	Metres	\$220	\$165,000	\$49,500	\$32,175	\$250,000
	1	1200mm Gravity Sewer	CONC	750	Metres	\$2,400	\$1,800,000	\$540,000	\$351,000	\$2,700,000
		Pavement Rehabilitation	PVC	750	Metres	\$1,030	\$772,500	\$231,750	\$150,638	\$1,160,000
					Upgrad	de 1 Sub-Total:	\$2,737,500	\$821,250	\$533,813	\$4,110,000
	2	600mm Gravity Sewer Removal	CONC	958	Metres	\$130	\$124,540	\$37,362	\$24,285	\$190,000
Concept 1	2	675mm Gravity Sewer	PVC	958	Metres	\$1,160	\$1,111,280	\$333,384	\$216,700	\$1,670,000
					Upgrad	de 2 Sub-Total:	\$1,235,820	\$370,746	\$240,985	\$1,860,000
	6	200mm Gravity Sewer Removal	PVC	114	Metres	\$90	\$10,260	\$3,078	\$2,001	\$20,000
		300mm Gravity Sewer	PVC	114	Metres	\$620	\$70,680	\$21,204	\$13,783	\$110,000
		Pavement Rehabilitation	PVC	114	Metres	\$1,030	\$117,420	\$35,226	\$22,897	\$180,000
			\$198,360	\$59,508	\$38,680	\$310,000				
				Futı	ure Upgrades C	oncept 1 Total:	\$4,171,680	\$1,251,504	\$813,478	\$6,280,000
		900mm Gravity Sewer Removal	CONC	750	Metres	\$220	\$165,000	\$49,500	\$32,175	\$250,000
	1	1200mm Gravity Sewer	CONC	750	Metres	\$2,400	\$1,800,000	\$540,000	\$351,000	\$2,700,000
Concept 2		Pavement Rehabilitation	PVC	750	Metres	\$1,030	\$772,500	\$231,750	\$150,638	\$1,160,000
Concept 2					Upgrad	de 1 Sub-Total:	\$2,737,500	\$821,250	\$533,813	\$4,110,000
	2	600mm Gravity Sewer Removal	CONC	958	Metres	\$130	\$124,540	\$37,362	\$24,285	\$190,000
	2	675mm Gravity Sewer	PVC	958	Metres	\$1,160	\$1,111,280	\$333,384	\$216,700	\$1,670,000
					Upgrad	de 2 Sub-Total:	\$1,235,820	\$370,746	\$240,985	\$1,860,000
			\$3.973.320	\$1.191.996	\$774,797	\$5.970.000				

#### Table H.4: Cost Estimates for Recommended Upgrades under Growth Conditions

<sup>1</sup> Costs herein are comparable to other municipalities. Costs are representative of 2024 dollars. Unit costs have been rounded to the nearest \$10, <sup>2</sup> The total cost has been rounded up to the nearest \$10,000.



#### Table H.5: Cost Estimates for Future Servicing

Horizon	ID	ltem	Material	Quantity	Unit	Unit Cost <sup>1</sup>	Sub-Total	Contingency (30%)	Engineering (15%)	Total Cost <sup>2</sup>
		450mm Gravity Sewer	PVC	449	Metres	\$870	\$390,630	\$117,189	\$76,173	\$590,000
	7,24,25	600mm Gravity Sewer	PVC	350	Metres	\$960	\$336,000	\$100,800	\$65,520	\$510,000
		675mm Gravity Sewer	PVC	617	Metres	\$1,160	\$715,720	\$214,716	\$139,565	\$1,080,000
						Total Cost:	\$1,442,350	\$432,705	\$281,258	\$2,180,000
	40.00.00.04.44	300mm Gravity Sewer	PVC	928	Metres	\$620	\$575,360	\$172,608	\$112,195	\$870,000
5-Year	18,23,28,34,41	1200mm Gravity Sewer	CONC	1,621	Metres	\$2,400	\$3,890,400	\$1,167,120	\$758,628	\$5,820,000
		<b>i</b>		•		Total Cost:	\$4,465,760	\$1.339.728	\$870.823	\$6.690.000
	30	450mm Gravity Sewer	PVC	452	Metres	\$870	\$393,240	\$117,972	\$76,682	\$590,000
		, ,	•	•		Total Cost:	\$393.240	\$117.972	\$76.682	\$590.000
	31	1200mm Gravity Sewer	CONC	912	Metres	\$2,400	\$2,188,800	\$656,640	\$426.816	\$3,280,000
		,	<u> </u>		1	Total Cost:	\$2,188,800	\$656,640	\$426.816	\$3.280.000
	13	375mm Gravity Sewer	PVC	602	Metres	\$770	\$463,540	\$139.062	\$90.390	\$700.000
			1			Total Cost:	\$463,540	\$139.062	\$90,390	\$700.000
	27	1200mm Gravity Sewer	CONC	904	Metres	\$2,400	\$2,169,600	\$650,880	\$423.072	\$3,250,000
				1		Total Cost:	\$2,169,600	\$650,880	\$423.072	\$3,250,000
10-Year	29	600mm Gravity Sewer	PVC	559	Metres	\$960	\$536 640	\$160,992	\$104 645	\$810,000
						Total Cost:	\$536,640	\$160,992	\$104.645	\$810,000
	20.26.East									
	Trunk 1	1200mm Gravity Sewer	PVC	4,009	Metres	\$2,400	\$9,621,600	\$2,886,480	\$1,876,212	\$14,390,000
						Total Cost:	\$9,621,600	\$2,886,480	\$1,876,212	\$14,390,000
	-	375mm Gravity Sewer	PVC	324	Metres	\$770	\$249,480	\$74,844	\$48,649	\$380,000
	4,19,21	750mm Gravity Sewer	PVC	844	Metres	\$1,350	\$1,139,400	\$341,820	\$222,183	\$1,710,000
		900mm Gravity Sewer	PVC	164	Metres	\$2,090	\$342,760	\$102,828	\$66,838	\$520,000
					-	Total Cost:	\$1,731,640	\$519,492	\$337,670	\$2,610,000
	17	450mm Gravity Sewer	PVC	358	Metres	\$870	\$311,460	\$93,438	\$60,735	\$470,000
					-	Total Cost:	\$311,460	\$93,438	\$60,735	\$470,000
	3	450mm Gravity Sewer	PVC	442	Metres	\$870	\$384,540	\$115,362	\$74,985	\$580,000
						Total Cost:	\$384,540	\$115,362	\$74,985	\$580,000
	35.36	525mm Gravity Sewer	PVC	550	Metres	\$910	\$500,500	\$150,150	\$97,598	\$750,000
	33,30	675mm Gravity Sewer	PVC	175	Metres	\$1,160	\$203,000	\$60,900	\$39,585	\$310,000
20-Year						Total Cost:	\$703,500	\$211,050	\$137,183	\$1,060,000
	32	450mm Gravity Sewer	PVC	439	Metres	\$870	\$381,930	\$114,579	\$74,476	\$580,000
						Total Cost:	\$381,930	\$114,579	\$74,476	\$580,000
	2,33	375mm Gravity Sewer	PVC	421	Metres	\$770	\$324,170	\$97,251	\$63,213	\$490,000
						Total Cost:	\$324,170	\$97,251	\$63,213	\$490,000
		450mm Gravity Sewer	PVC	575	Metres	\$870	\$500,250	\$150,075	\$97,549	\$750,000
		675mm Gravity Sewer	PVC	1,096	Metres	\$1,160	\$1,271,360	\$381,408	\$247,915	\$1,910,000
	0,1,11,14,15,22	750mm Gravity Sewer	PVC	797	Metres	\$1,350	\$1,075,950	\$322,785	\$209,810	\$1,610,000
		900mm Gravity Sewer	PVC	829	Metres	\$2,090	\$1,732,610	\$519,783	\$337,859	\$2,600,000
		1050mm Gravity Sewer	PVC	1,292	Metres	\$2,250	\$2,907,000	\$872,100	\$566,865	\$4,350,000
					•	Total Cost:	\$7,487,170	\$2,246,151	\$1,459,998	\$11,220,000
	6	200mm Forcemain	PVC	902	Metres	\$540	\$487,080	\$146,124	\$94,981	\$730,000
						Total Cost:	\$487,080	\$146,124	\$94,981	\$730,000
	8	450mm Gravity Sewer	PVC	296	Metres	\$870	\$257,520	\$77,256	\$50,216	\$390,000
					•	Total Cost:	\$257,520	\$77,256	\$50,216	\$390,000
	5	525mm Gravity Sewer	PVC	298	Metres	\$910	\$271,180	\$81,354	\$52,880	\$410,000
					•	Total Cost:	\$271,180	\$81,354	\$52,880	\$410,000
		450mm Gravity Sewer	PVC	812	Metres	\$870	\$706,440	\$211,932	\$137,756	\$1,060,000
	9,38,39,40,42	600mm Gravity Sewer	PVC	1,562	Metres	\$960	\$1,499,520	\$449,856	\$292,406	\$2,250,000
Ultimate		750mm Gravity Sewer	PVC	81	Metres	\$1,350	\$109,350	\$32,805	\$21,323	\$170,000
						Total Cost:	\$2,315,3 <u>10</u>	\$694,59 <u>3</u>	\$451,485	\$3,480,000
	12	600mm Gravity Sewer	PVC	253	Metres	\$960	\$242,880	\$72,864	\$47,362	\$370,000
						Total Cost:	\$242.880	\$72.864	\$47,362	\$370.000
	10.10	450mm Gravity Sewer	PVC	184	Metres	\$870	\$160,080	\$48,024	\$31,216	\$240,000
	10,16	600mm Gravity Sewer	PVC	372	Metres	\$960	\$357.120	\$107.136	\$69.638	\$540.000
					•	Total Cost:	\$517.200	\$155,160	\$100.854	\$780.000
	N/A	Lift Station 1 (31 L/s)	N/A	1	Item	\$700.000	\$700.000	\$210,000	\$136,500	\$1,050,000
		·····			•	Total Cost:	\$700,000	\$210,000	\$136,500	\$1,050,000

<sup>1</sup> Costs herein are comparable to other municipalities. Costs are representative of 2024 dollars. Unit costs have been rounded to the nearest \$10.
<sup>2</sup> The total cost has been rounded up to the nearest \$10,000.